

#### BRIAN TAYLOR STATE FIRE MARSHAL

January 15, 2025

Mr. Mathew Griffith, AIA in situ studio 704 N. Person Street Raleigh, NC 27604

RE: 2018 NCRC Section R309.2 Carports

Mr. Griffith:

This letter is in response to your request for a formal interpretation from the Office of State Fire Marshal ("OSFM") dated 2/23/24 and received by OSFM the same day. This delayed formal response letter serves to complete the documentation requested by Mathew Griffith of in situ studio. Thank you for your patience in waiting for this formal interpretation response while our office handled other commitments. Requests are addressed below in the order in which they are posed.

Stated in relevant parts:

"Requesting review of permit review decision by Griffin Todd the City of Raleigh that the carport as designed (REF to attached drawings) is no "open on at least two sides" and is, thus, considered a garage.

The carport as designed has a 6' wide garden to the left side that is open to the sky by a hole in the roof. It was our intent that this would fill the open requirement.

Further, if it is DOI's opinion the carport as designed cannot be considered open on two sides, please confirm the scope of garage requirements that come into play. The language of the code refers to "comply[ing] with the provisions of this section for garages." Section 309 garage requirements only require slope and a noncombustible parking surface. It is unclear that any other requirements for garages would need to be met."

#### Remarks:

Attachment A is comprised of the request for formal interpretation as well as all supporting information submitted with the request.



**Code Analysis**: Carports shall be open on not less than two sides otherwise, carports shall be considered garages as noted in 2018 North Carolina Residential Code, Section R309.2.

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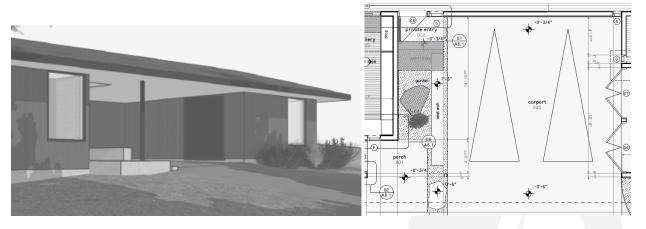
**R309.2 Carports.** Carports shall be open on not less than two sides. Carport floor surfaces shall be of *approved* noncombustible material. Carports not open on two or more sides shall be considered to be a garage and shall comply with the provisions of this section for garages.

**Exception:** Asphalt surfaces shall be permitted at ground level in carports.

The area of floor used for parking of automobiles or other vehicles shall be sloped to facilitate the movement of liquids to a drain or toward the main vehicle entry doorway

....

**Conclusions:** A careful review of the design drawings in Attachment A indicate one open side on the front elevation that extends the entire length of the opening of the space where the parking of automobiles or other vehicles will occur plus the space allocated for the porch, seat wall, garden and private entry. A separate opening to sky is also provided above the garden and seat wall near the private entry. Both the front elevation side opening and the opening to sky do not have any doors.



The code is silent on the configuration of the two open sides required to classify a space used for parking automobiles or other vehicles as a carport. The opening to sky is a separate opening from the front elevation side opening and therefore is considered the second opening required to classify the space called a "carport" in the Attachment A design drawings as a carport in the context in the application of the 2018 North Carolina State Building Code.



Sincerely,

David Rittlinger, PE, LEED AP

DR. Rittlinger

Division Chief – Codes & Interpretations North Carolina Office of State Fire Marshal

cc: Nathan Childs, NCDOJ, counsel for NC Building Code Council, <a href="mailto:nchilds@ncdoj.gov">nchilds@ncdoj.gov</a>

Nicki Shaffer, NCDOJ, counsel for NC Residential Code Council, wshaffer@ncdoj.gov

Pak Yip, NCOSFM, Chief Code Consultant, <a href="mailto:pak.yip@ncdoi.gov">pak.yip@ncdoi.gov</a>



#### **ATTACHMENT A**

(see attached pdf)



### APPENDIX E APPEALS NORTH CAROLINA BUILDING CODE COUNCIL

1429 Rock Quarry Road, Suite 105 Raleigh, North Carolina 27610 (919) 647-0008

david.rittlinger@ncdoi.gov

APPEAL TO NCDOI/NCBCO GS 153A-374, GS 160A-434 Formal Interpretation by NCDOI Appeal of Local Decision to NCDOI	GS 143-140, GS 143-141 Appeal of Local Decision to NCBCC Appeal of NCDOI Decision to NCBCC
REPRESENTING Ammarah and Asad Abba	
CITY Raleigh	STATE NC ZIP 27612
E-MAIL matt@insitustudio.us	FAX ( )
North Carolina State Building Code, Volume R	- Section <u>309.2</u>
REQUEST ONE: [ ] Formal Interpretation by N [x ] Appeal of Local Decision to	ICDOI [ ] Appeal of Local Decision to NCBCC to NCDOI [ ] Appeal of NCDOI Decision to NCBCC
Type or print. Include all background information as attached policies. Attach additional supporting information and action and action and action and action and action and action actions.	required by the referenced General Statutes and the rmation.
Requesting review of permit review decision by Griffin To attached drawings) is no "open on at least two sides" and	odd the City of Raleigh that the carport as designed (REF to a land is, thus, considered a garage.
The carport as designed has a 6' wide garden to the left that this would fill the open requirement.	side that is open to the sky by a hole in the roof. It was our intent
garage requirements that come into play. The language	nnot be considered open on two sides, please confirm the scope of of the code refers to "comply[ing] with the provisions of this only require slope and a noncombustible parking surface. It is need to be met.
Thanks!	
REASON:	APPEAL TO NCDOI/NCBCC
Signature	DATE: 02 23 24 FORM 3/14/17

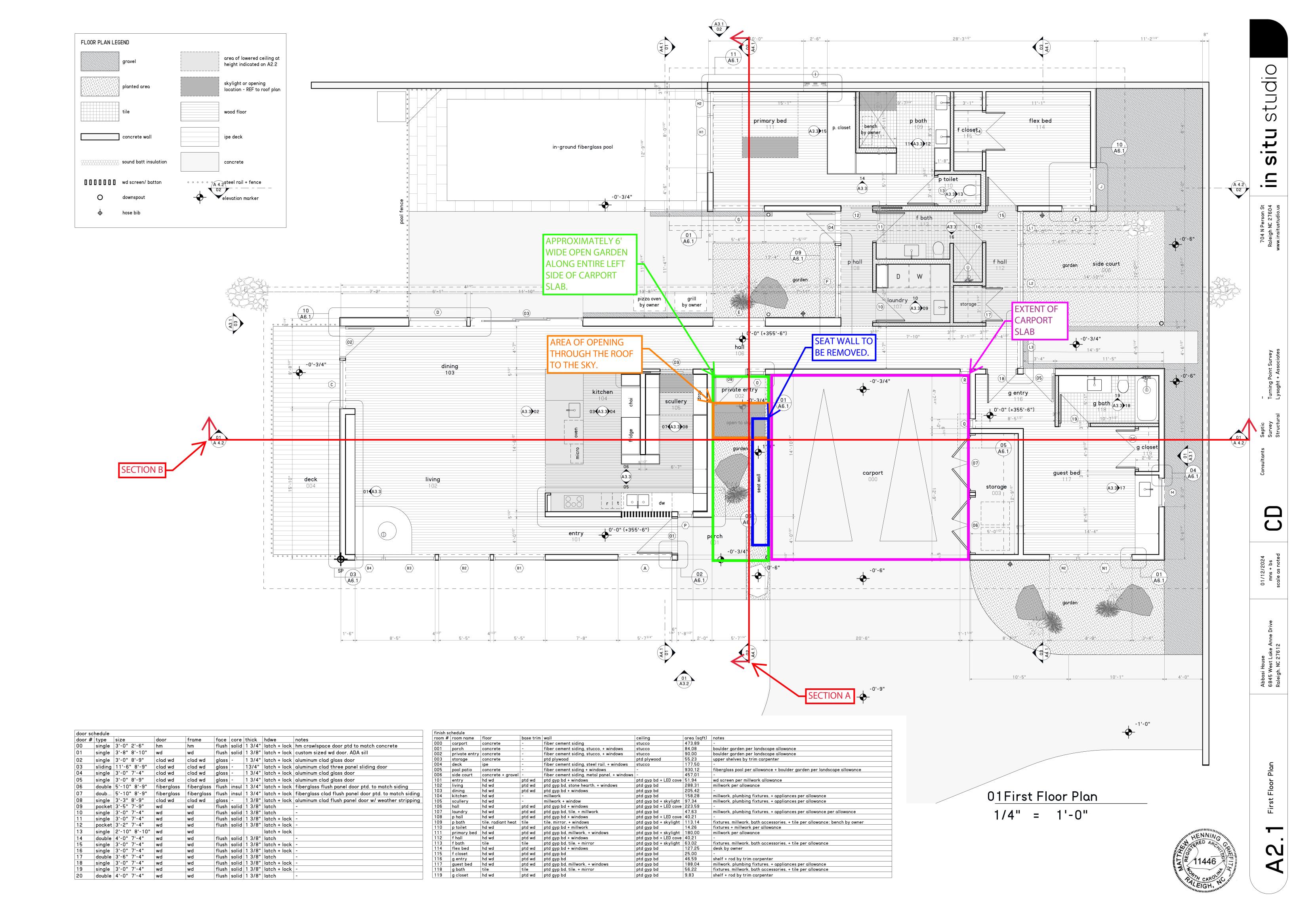
#### 202.9 Appeals

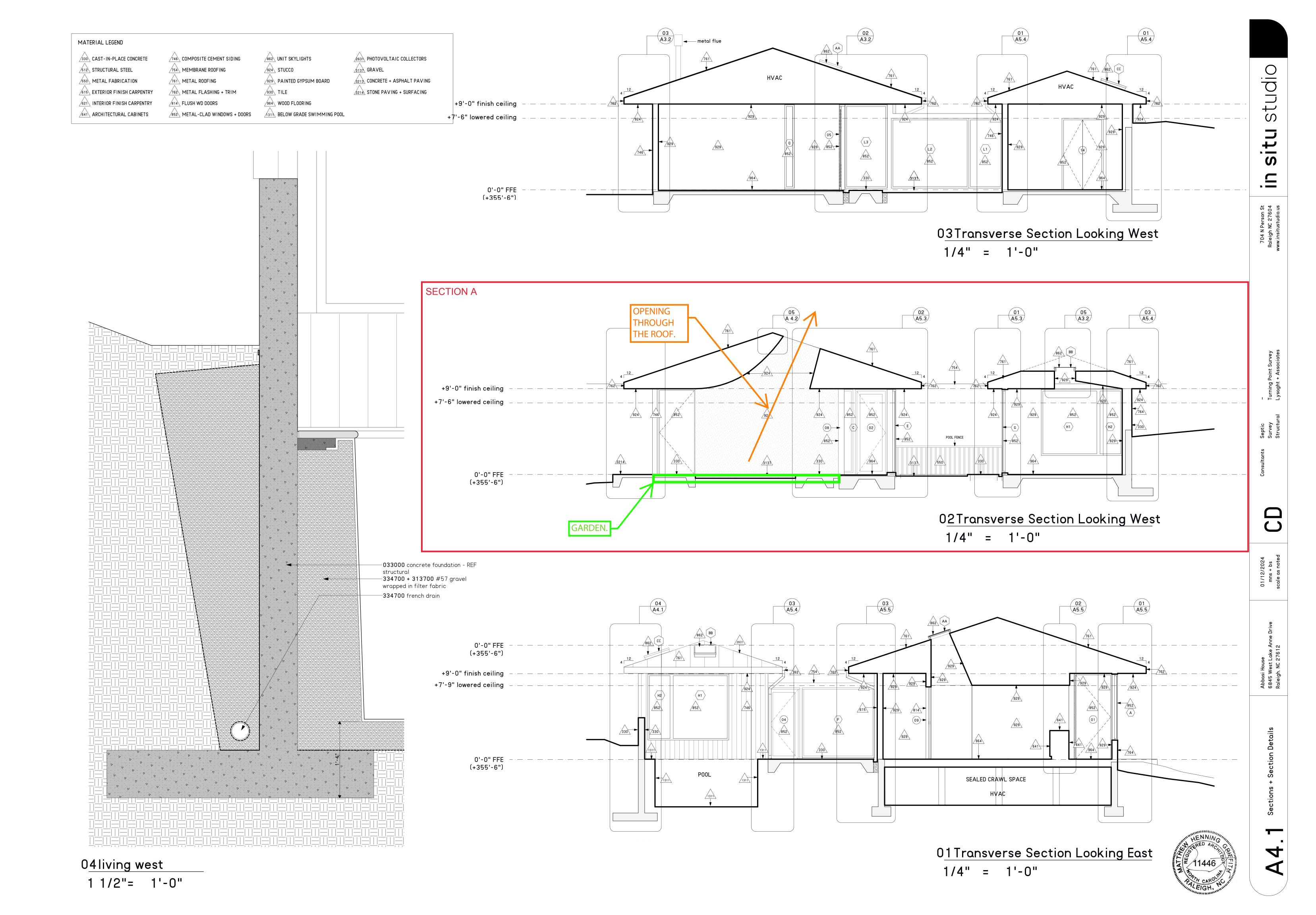
**202.9.1 Engineering Division.** A written technical interpretation shall be provided as specified in Section 203.2.1.2. Any person may appeal in writing an order, decision, or determination pertaining to the code or any state building law by filing written notice with the Commissioner of Insurance or his designee within ten (10) days after the order, decision, or determination. A copy of the appeal shall be furnished to each party. (General Statutes 143-140, 153A-374 and 160A-434)

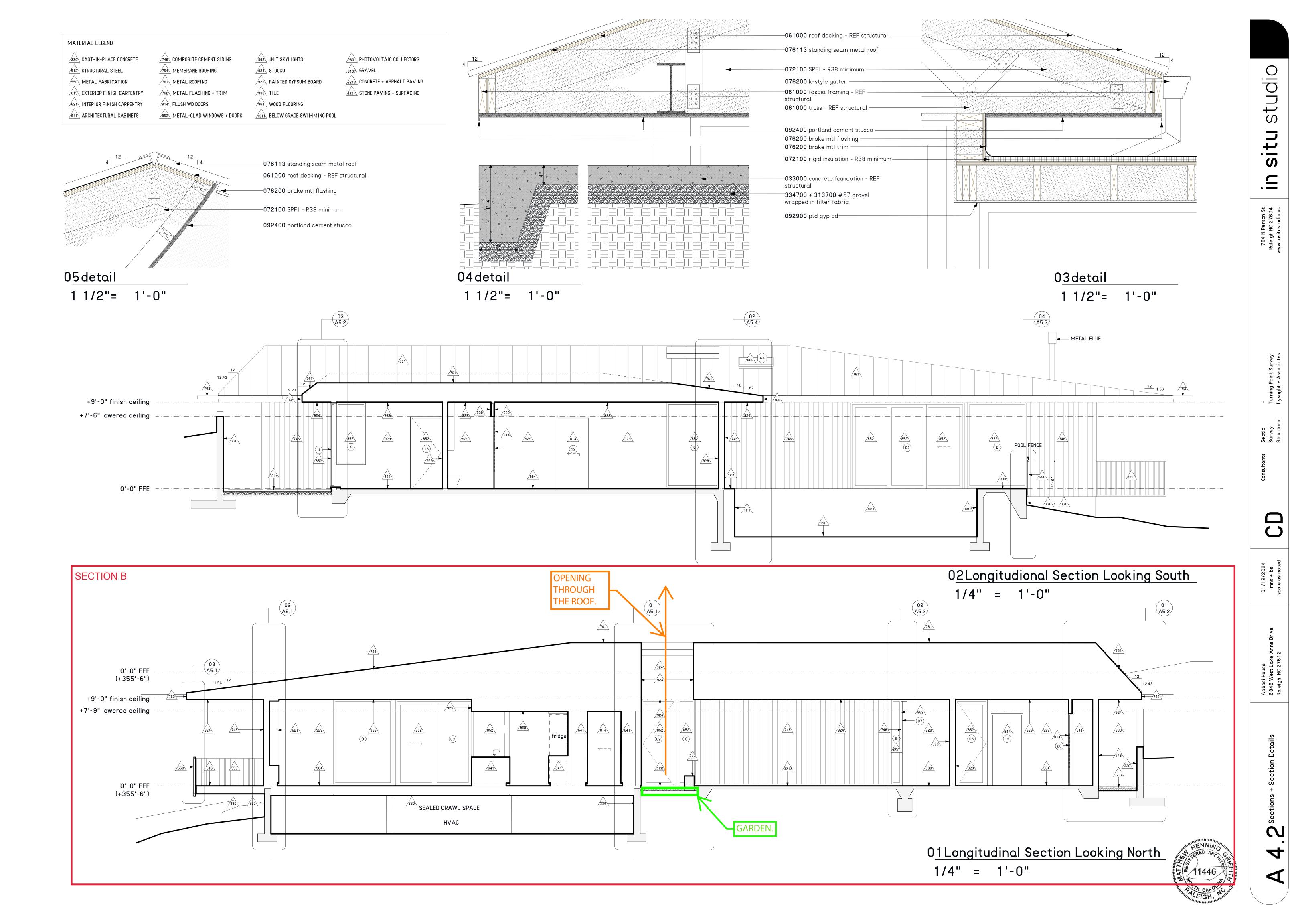
#### 203.2.1 Interpretations

- **203.2.1.1 Informal Interpretations.** The Engineering Division shall provide informal interpretations on code related matters either by e-mail, letter or telephone. These informal interpretations may be accepted by the local code enforcement official or party requesting the interpretation. Either party may request a formal interpretation of the code.
- **203.2.1.2 Formal Interpretations.** Any person may request in writing a formal interpretation of the code. The request shall be addressed to the Chief Code Consultant for the Department of Insurance. The request shall be specific and shall reference the code sections in question. All formal interpretations shall be in writing. A formal interpretation shall be binding on all parties unless appealed to the Building Code Council as specified in Section 201.9.2. Formal interpretations determined to be of a general nature may be posted on the Department website. (General Statute 143-140)
- **203.2.2 Appeals.** Any person may appeal in writing an order, decision, or determination of a code enforcement official pertaining to the code or any state building law. The appeal shall be addressed to the Chief Engineer for the Department of Insurance by filing written notice within ten (10) days after the order, decision, or determination. The appeal shall contain the type and size of the building in question, the location of the building, and shall reference the code sections in question. The decision shall be in writing and shall set forth the facts found. The decision rendered shall be based on the technical provisions of the code, public health and safety and shall be construed liberally to those ends. A decision shall be binding on all parties unless appealed to the Building Code Council as specified in Section 201.9.2. A copy of the appeal and written decision shall be furnished to each party. (General Statutes 153A-374 and 160A-434)
- **202.9.2 Building Code Council.** The Building Code Council shall hear appeals from the decisions of State enforcement agencies relating to any matter related to the code. Any person wishing to appeal a decision of a State enforcement agency to the Building Code Council shall give written notice of appeal as follows:
  - **202.9.2.1** Twenty one (21) copies including an original of the Notice of Appeal shall be filed with the Building Code Council c/o NC Department of Insurance, Engineering Division, 325 North Salisbury Street, Room 5\_44, Raleigh, NC 27603 and one (1) copy shall be filed with the State enforcement agency from which the appeal is taken.
  - **202.9.2.2** The Notice of Appeal shall be received no later than thirty (30) days from the date of the decision of the State enforcement agency.

- **202.9.2.3** The Notice of Appeal shall be legibly printed, typewritten or copied and shall contain the following:
  - (1) Name, address of the party or parties requesting the appeal.
  - (2) The name of the State enforcement agency, the date of the decision from which the appeal is taken, and a copy of the written decision received from the enforcement agency.
  - (3) The decision from which the appeal is taken shall be set forth in full in the Notice of Appeal or a copy of the decision shall be attached to all copies of the Notice of Appeal.
  - (4) The contentions and allegations of fact must be set forth in full in a clear and concise manner with reference to the sections of the code in controversy.
  - (5) The original Notice of Appeal shall be signed by the party or parties filing appeal.
  - (6) The Notice of Appeal shall be received by the first day of the month prior to the Building Code Council's quarterly scheduled meeting in order to be placed on the agenda for that meeting. The Chairman may schedule a special meeting to hear an appeal.
- 202.9.2.4 Upon the proper filing of the Notice of Appeal, the Building Code Council Secretary shall forward one (1) copy of the Notice of Appeal to each member of the Building Code Council. The Chairman may appoint a Hearing Committee to hear appeals. The Secretary shall send notice in writing to the party or parties requesting an appeal and to the Building Code Council Hearing Committee members at least fifteen (15) days prior to the Hearing Committee meeting. A written decision of the Hearing Committee meeting shall be provided to all Building Code Council Members. The actions of the Hearing Committee shall be final, unless appealed to the full Building Code Council in writing within 30 days of the Hearing Committee's action. If a Hearing Committee consists of at least seven Council members, it will constitute a quorum of the full Council. Further appeals shall be as specified in Section 202.9.3.
- **202.9.2.5** The Building Code Council shall, upon a motion of the State enforcement agency or on its own motion, dismiss appeals for the following reasons:
  - (1) Not pursued by the appellant or withdrawn;
  - (2) Appeal not filed in accordance with these rules; or
  - (3) Lack of jurisdiction.
- **202.9.2.6** When the Building Code Council finds that a State enforcement agency was in error in its interpretation of the code, the Building Code Council shall remand the case to the agency with instructions to take such actions as the Building Code Council directs. When the Building Code Council finds on appeal that materials or methods of construction proposed are equivalent to those required by the code, the Building Code Council shall remand the case to the State enforcement agency with instructions to permit the use of such materials or methods of construction. The Building Code Council shall immediately initiate procedures for amending the code to permit the use of such materials or methods of construction.
- **202.9.2.7** The Building Code Council shall provide a written decision setting forth the findings of fact and the Building Code Council's conclusions to each party or parties filing the appeal and to the State enforcement agency from which the appeal was taken.
- **202.9.3 Superior Court.** Whenever any person desires to appeal a decision of the Building Code Council or a decision of a State or local enforcement agency, he may appeal either to the Wake County Superior Court or the Superior Court of the county in which the proposed building is to be situated in accordance with the provisions of Chapter 150B of the General Statutes. (General Statute 143-141(d))







### Abbasi House

Ammarah + Asad Abbasi 6843 West Lake Anne Drive Raleigh, NC 27612

#### PERMIT RESUBMISSION SET



INF0	R-2 CoR ZONING
	PIN 0777931356
ZONING	0.92 ACRES
Z0	

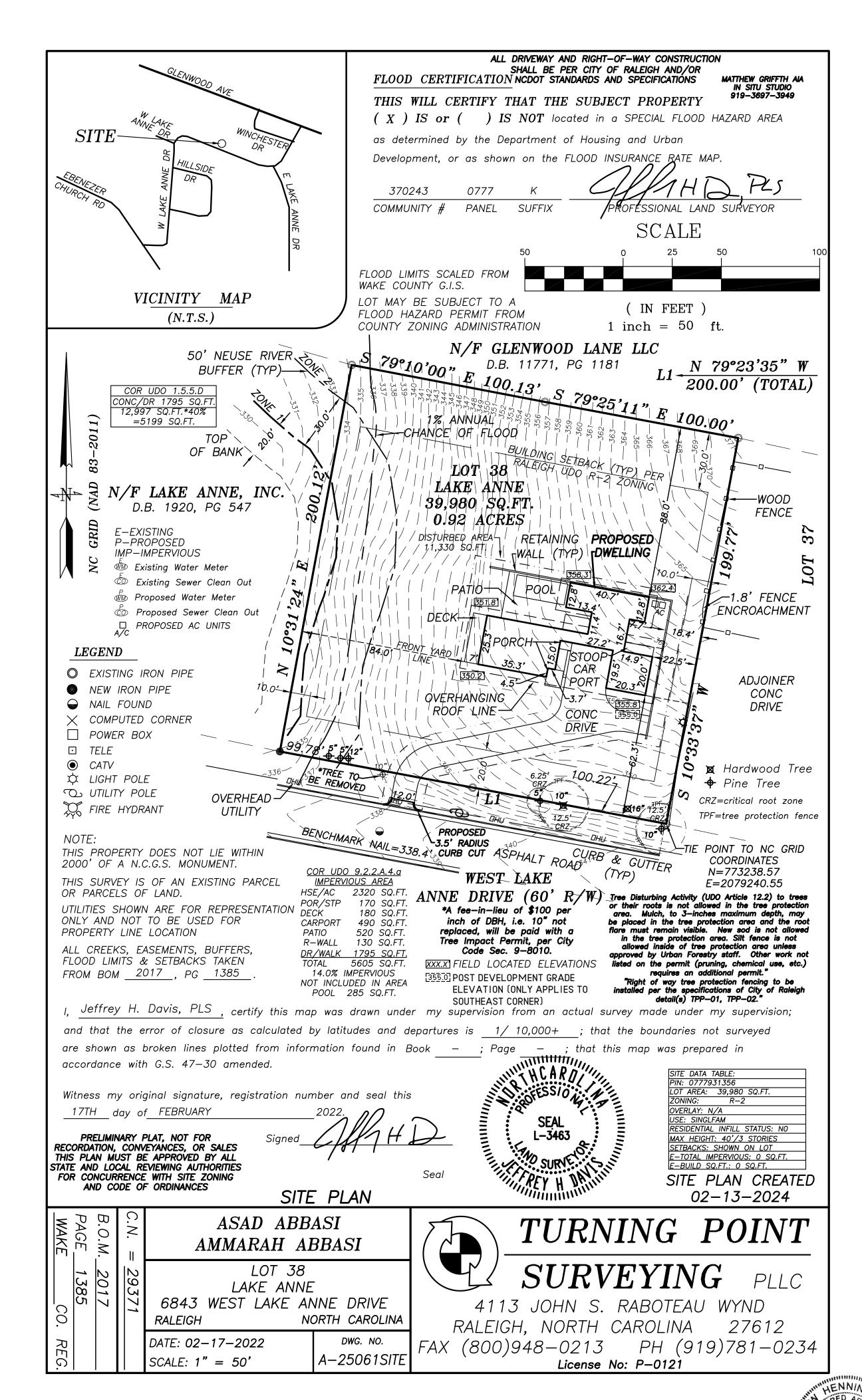
ZONING	R-2
LOT AREA	40075.2 SF
TOTAL IMPERVIOUS SF	7031 SF
TOTAL UNDER ROOF SF	4260 SF
GROUND FLOOR CONDITIONED SF	2269 SF
CARPORT	474 SF
UNHEATED CARPORT STORAGE	68 SF
CRAWLSPACE SF	972 SF
PATIO SF	633 SF
POOL SF	280 SF
INFILL STANDARDS DO NOT APPLY	

DRAWING LIST	SHEET#	SHEET NAME
9		
₹		
Æ	A0.0	Cover Sheet
冶	A1.0	Site Plan
	A1.1	Septic Layout
	A2.0	Foundation Plan
	A2.1	First Floor Plan
	A2.2	First Floor RCP
	A2.3	Roof Plan
	A3.1	Elevations
	A3.2	Elevation + Section Details
	A3.3	Interior Elevations + Window Schedule
	A4.1	Sections + Section Details
	A4.2	Sections + Section Details
	A5.1	Sections Details Sections Details
	A5.2 A5.3	Sections Details Sections Details
	A5.4	Sections Details Sections Details
	A5.5	Sections Details
	A6.1	Plan Details
	\$1.0	General Structural Notes
	\$1.1	Foundation and First Floor Framing Plan
	\$1.2	Roof Framing Plan
	\$1.3	Shear Wall Sections
	\$2.1	Foundation and Framing Details
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330 CAST-IN-PLACE CONCRETE	746 CEMENTIOUS SIDING + TRIM	855 VINYL WINDOWS	2631 PHOTOVOLTAIC COLLECTORS
512 STRUCTURAL STEEL	754 MEMBRANE ROOFING	924 STUCCO	3137 GRAVEL
METAL FABRICATION	731 ASPHALT SHINGLE ROOF	929 PAINTED GYPSUM BOARD	2213 CONCRETE + ASPHALT PAVING
615 EXTERIOR FINISH CARPENTRY	762 METAL FLASHING + TRIM	930 TILE	3214 STONE PAVING + SURFACING
621 INTERIOR FINISH CARPENTRY	814 FLUSH WD DOORS	964 WOOD FLOORING	862 UNIT SKYLIGHTS
641 ARCHITECTURAL CABINETS	METAL-CLAD WINDOWS + DOORS	BELOW GRADE SWIMMING POOL	

DEDUCT ALTERNATES	01	HardiePlank siding in lieu of Hardie Artisan siding
TERN	02	HardiePanel soffits in lieu of stucco soffits
F.	03	Asphalt shingle roof in lieu of standing seam metal roof
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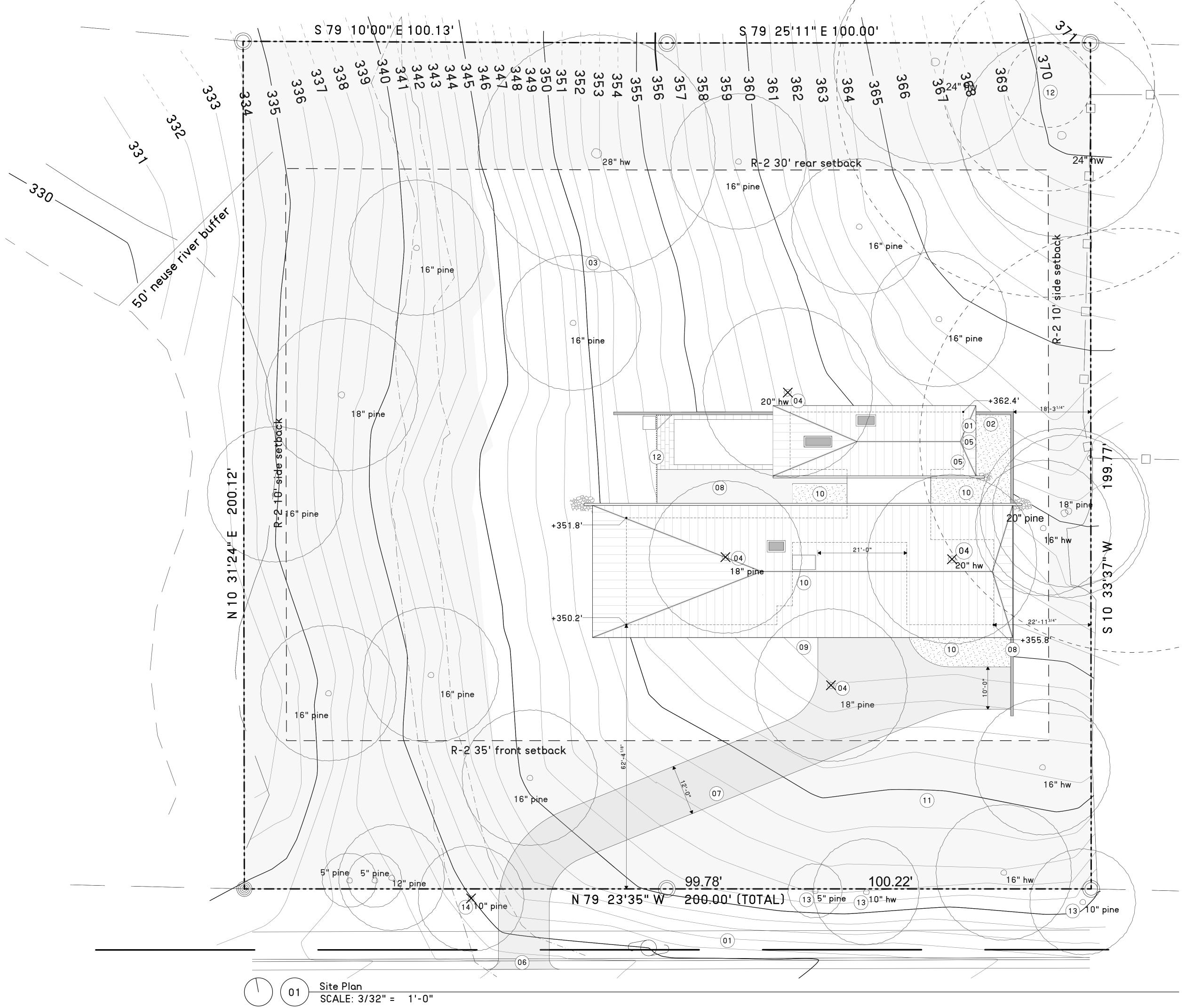
ALLOWANCES	A. APPLIANCES	\$16,000
0WA	B. DOOR HARDWARE	\$3.000
ALL	C. MILLWORK	\$48,000
	D. LIGHT FIXTURES (included cans)	\$14,000
	E. PLUMBING FIXTURES	\$14,000
	F. PORCELAIN TILE (materials only)	\$8 / SF
	G. STRUCTURED WIRING	\$5,000
	H. WINDOW TREATMENTS	\$8,000
	I. BATH ACCESSORIES	\$5,000
	J. COUNTERTOPS (materials only)	\$13,000
	K. PLANTING + BOULDERS	\$20,000
	L. STOVE	\$12,000
	M. POOL	\$50,000
	N. POOL SOLAR HOT WATER PANELS	\$15,000



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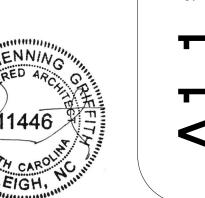
704 N Person St Raleigh NC 27604 www.insitustudio.us

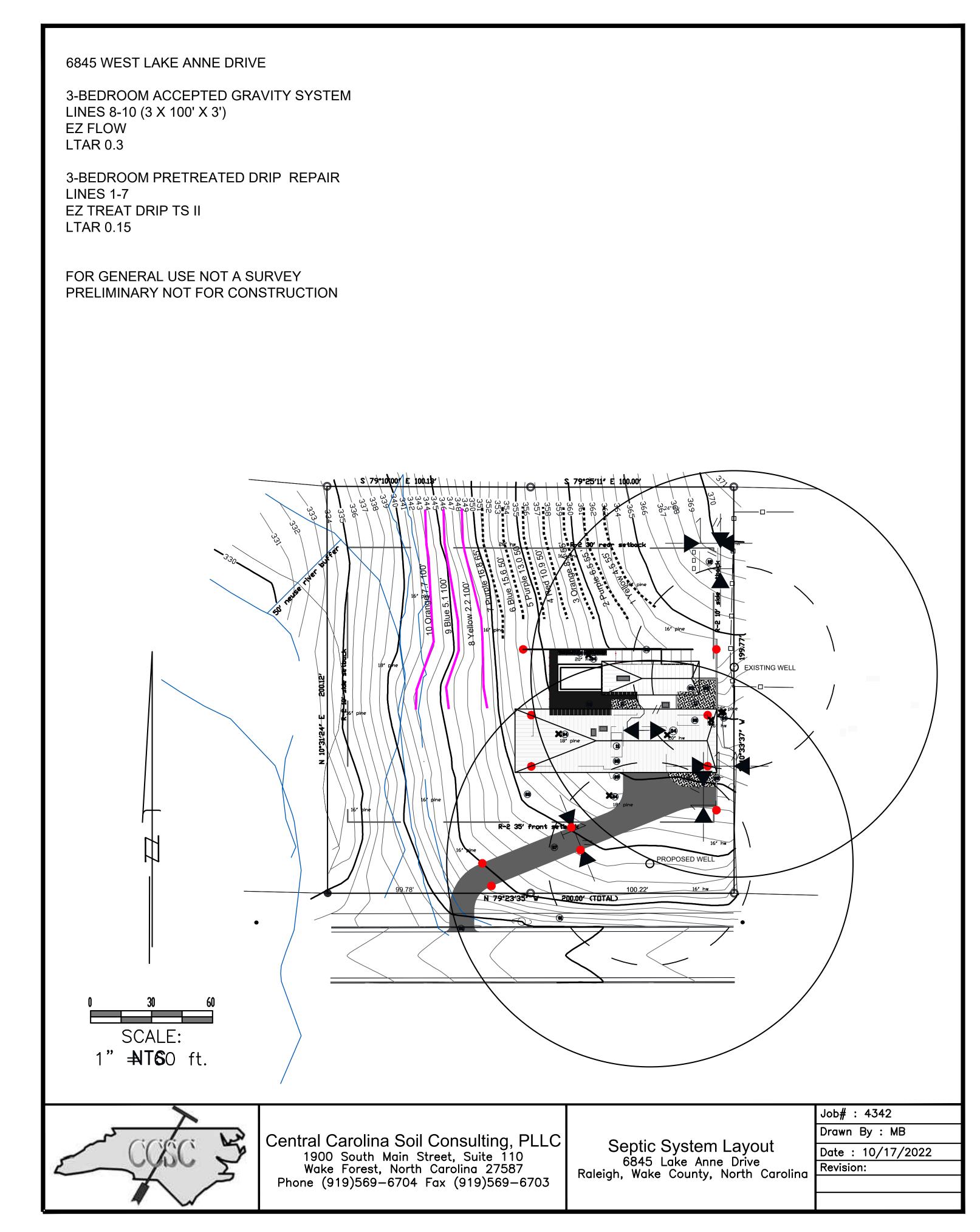


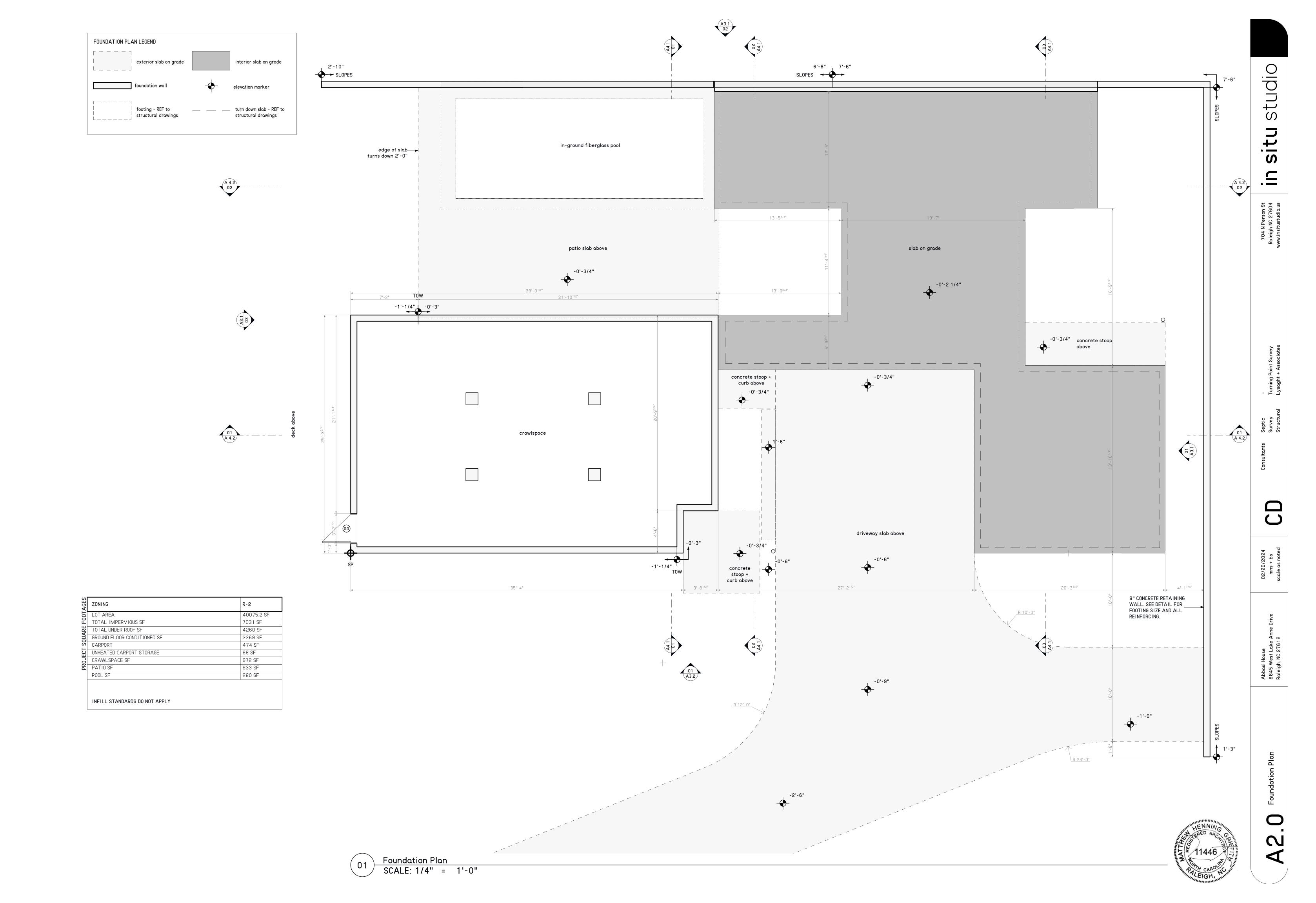
#### SITE NOTES LEGEND

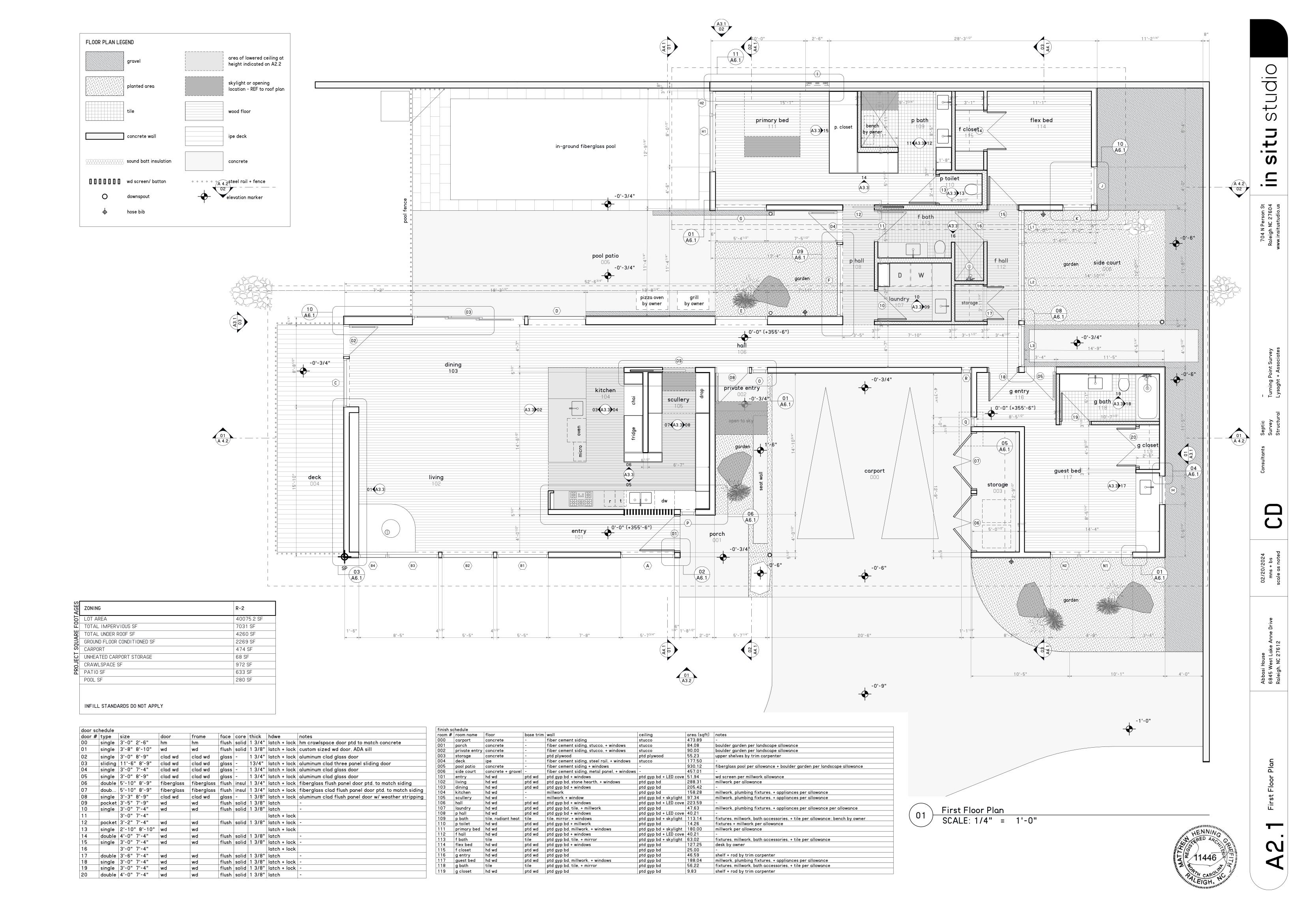
- O1 Provide 200A single phase electrical and cable services from existing utility locations, accessing the house under the new driveway.
- 02 Install exterior HVAC units and pool equipment, heat recovery, and heater per site plan and architect's direction.
- 03 New septic system
- O4 Trees to be removed, grind rootball. all other trees to remain.
- 05 Locate exterior cable/internet service per drawings and architect's direction.
- 06 New curb cut per city of Raleigh standard.
- 07 New concrete drive and parking.08 New concrete stoops, patio, and retaining walls.
- New concrete stoops, patio, and retain
- 09 New bluestone paver.
- 10 Landscape planting per allowance.
- 11 New well.
- 12 New pool fence.
- 13 Tree Disturbing Activity (UDO Article 12.2) to trees or their roots is not allowed in the tree protection area. Mulch, to 3-inches maximum depth, may be placed in the tree protection area and the root flare must remain visible. New sod is not allowed in the tree protection area. Silt fence is not allowed inside of tree protection area unless approved by Urban Forestry staff. Other work not listed on the permit (pruning, chemical use, etc.) requires an additional
- 14 Tree to be removed, grind rootball. Fee-in-lieu replacement at the rate of \$100 per inch of DBH applies to this tree. Fee will be \$1000.

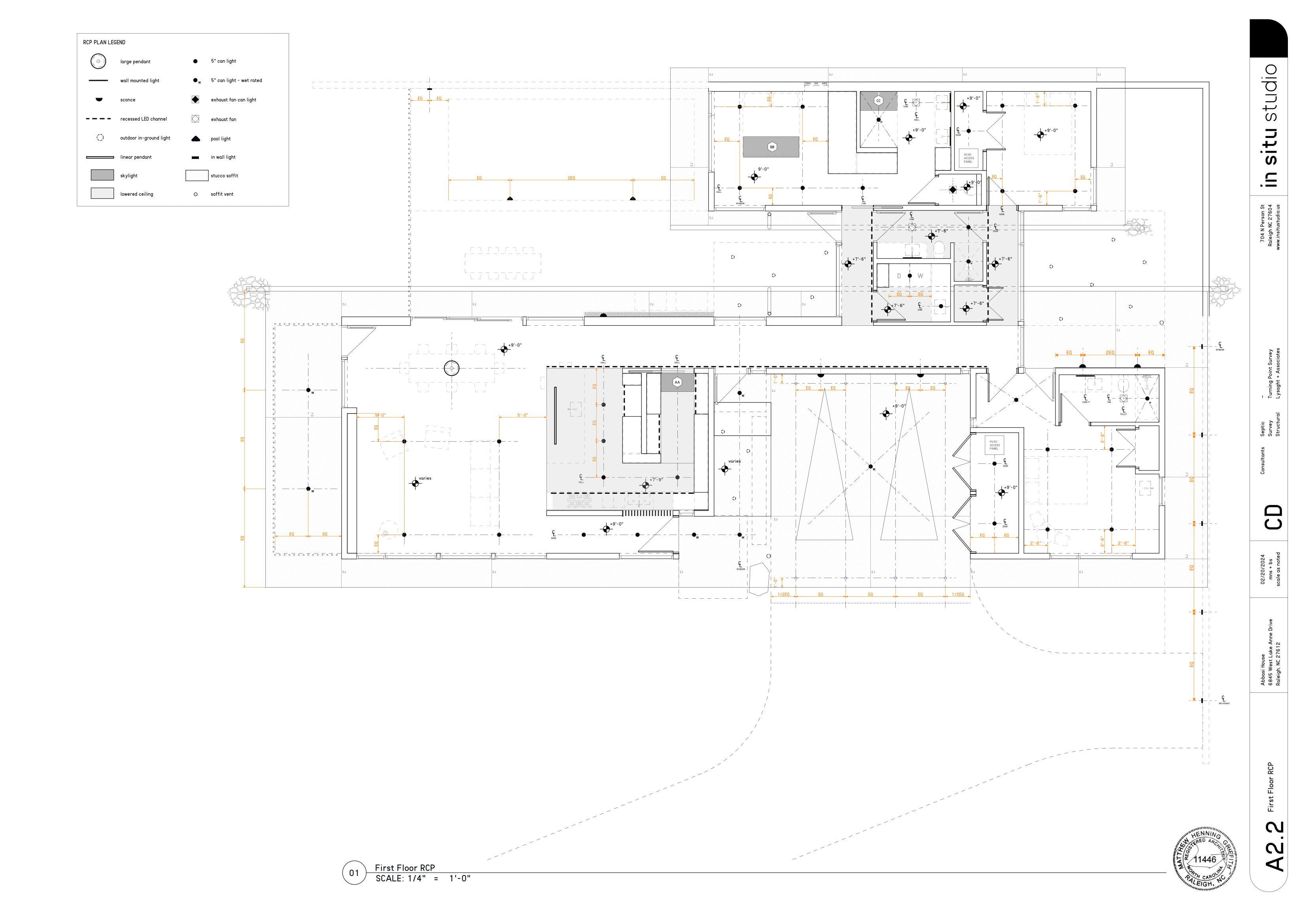
permit. Right of way tree protection fencing to be installed per the specifications of City of Raleigh detail(s) TPP-01, TPP-02.

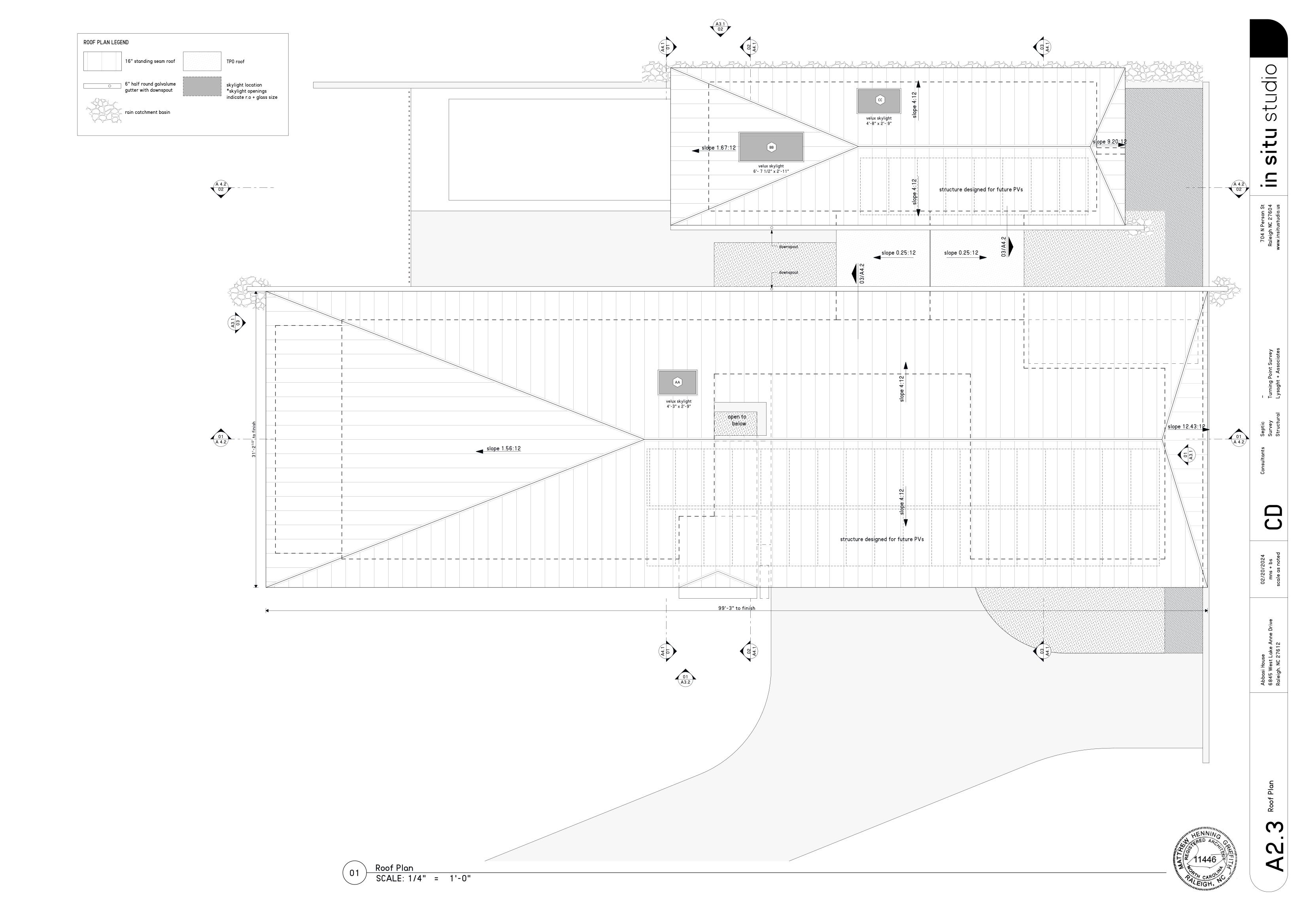


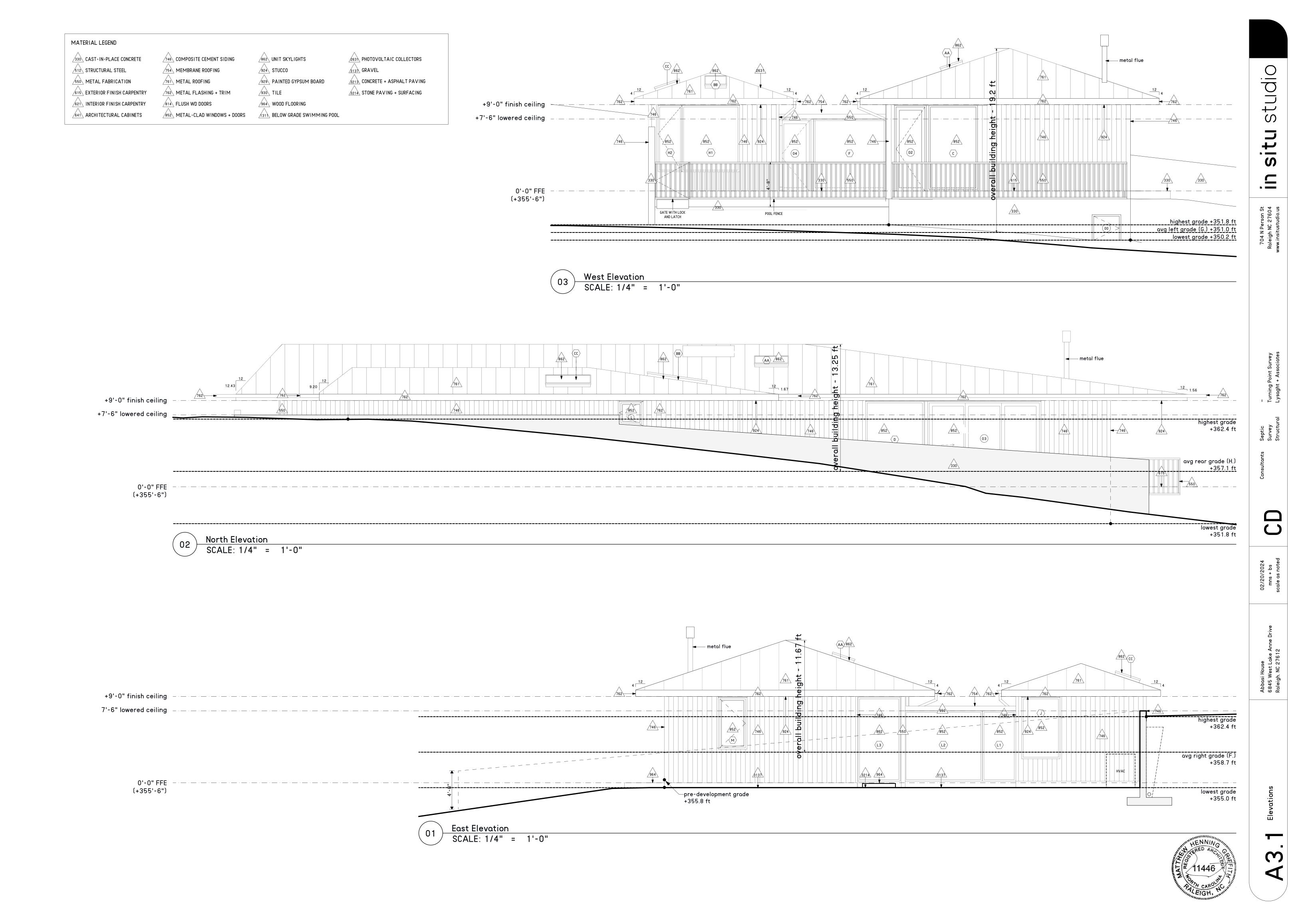


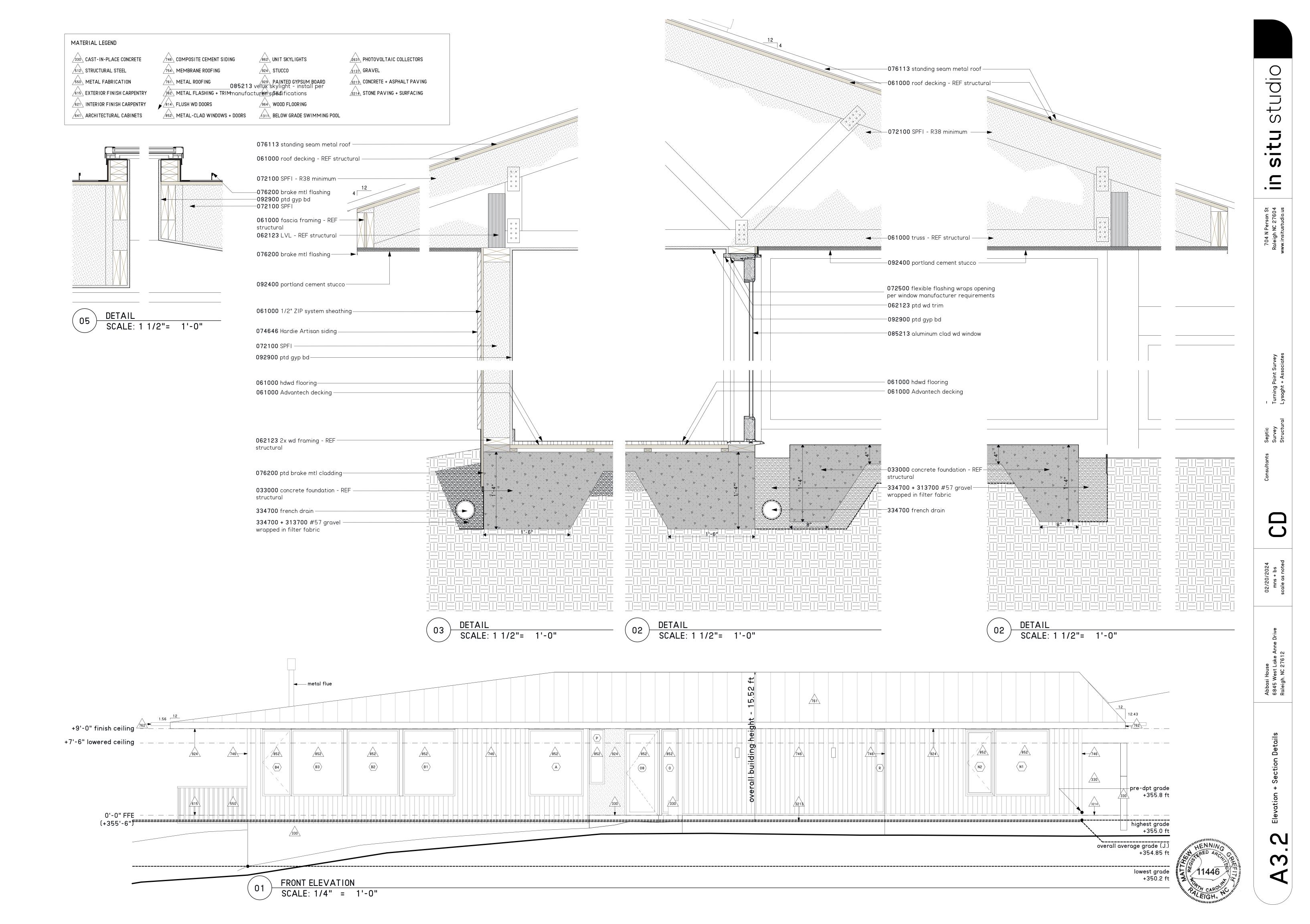




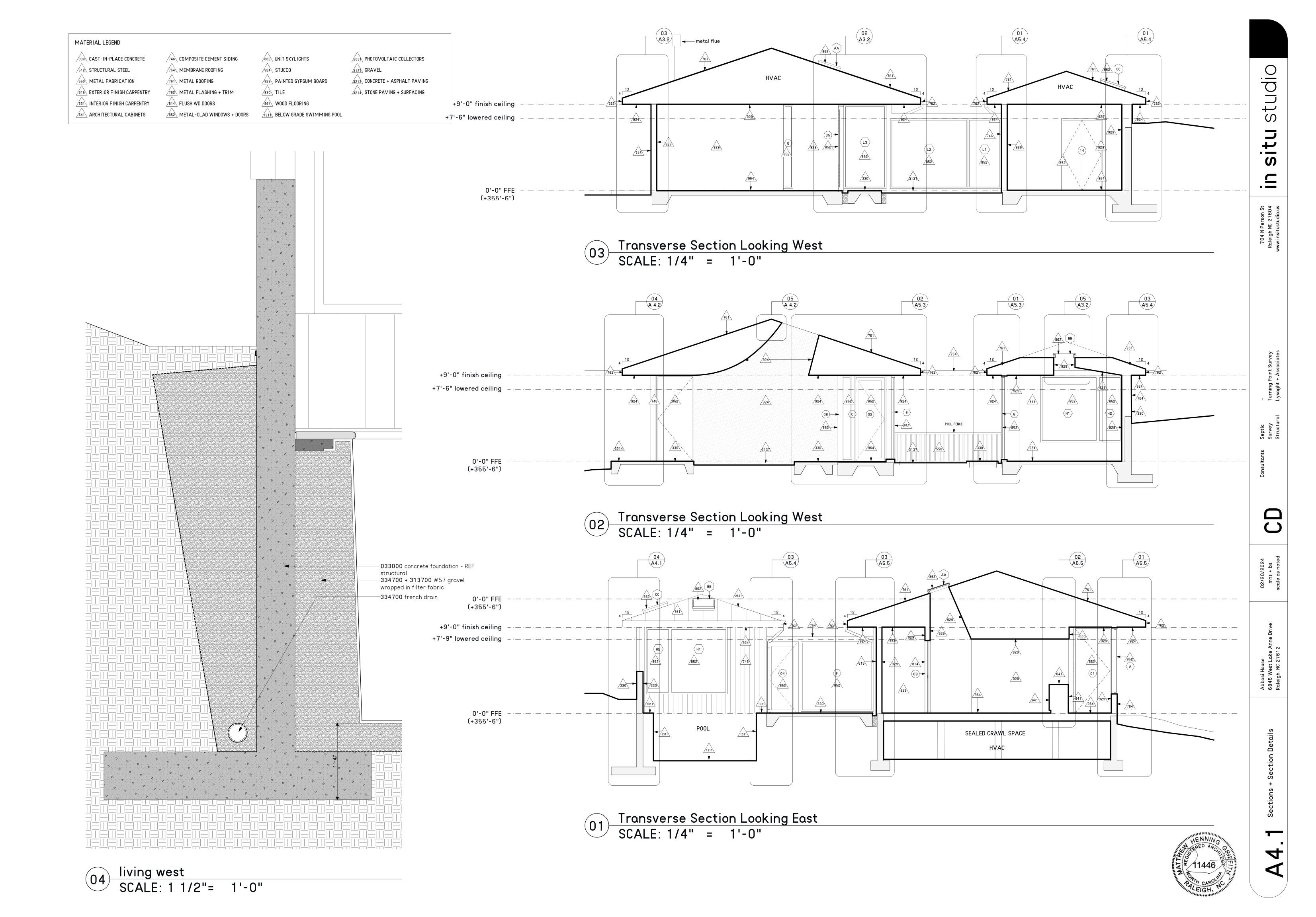


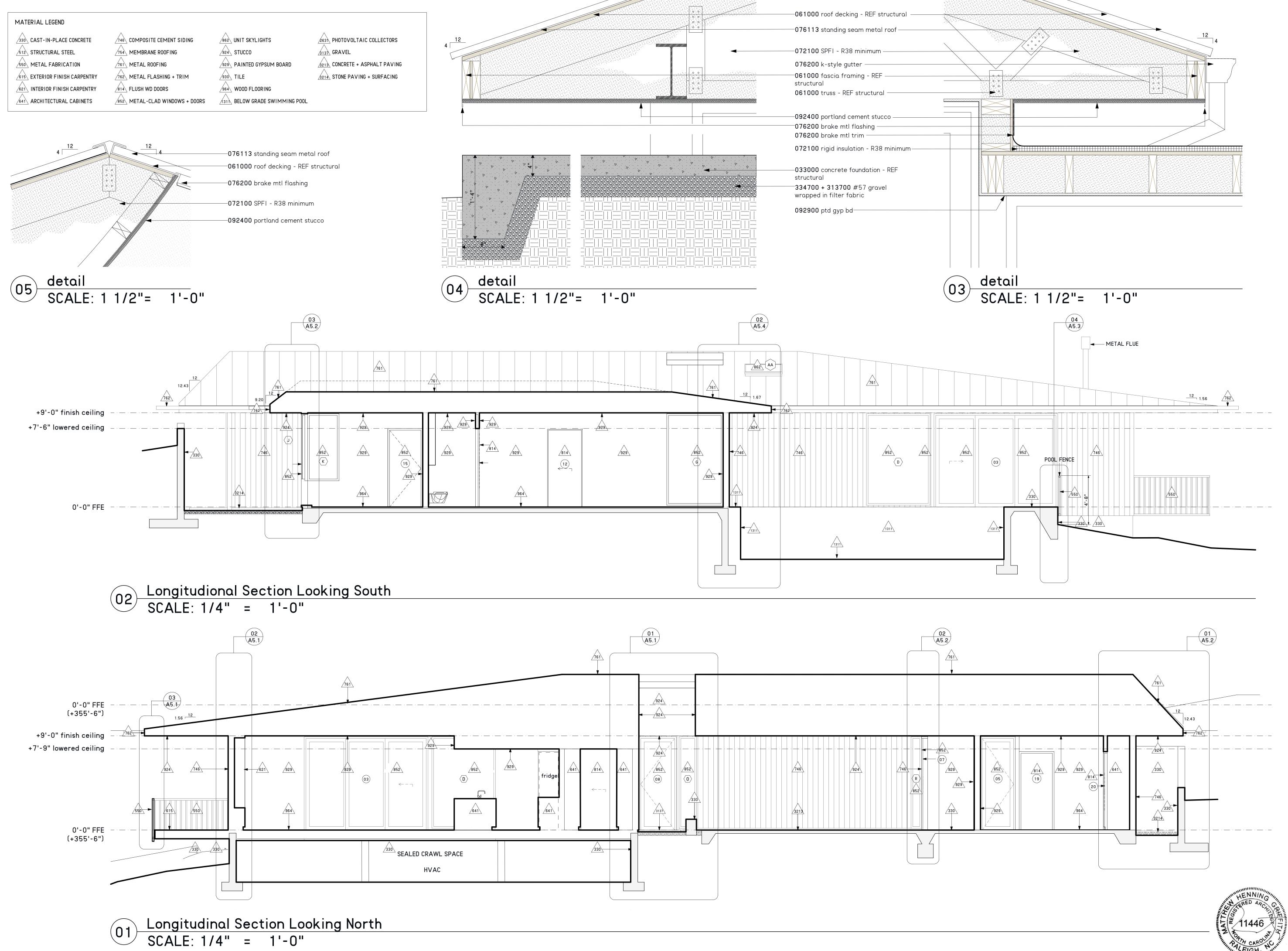












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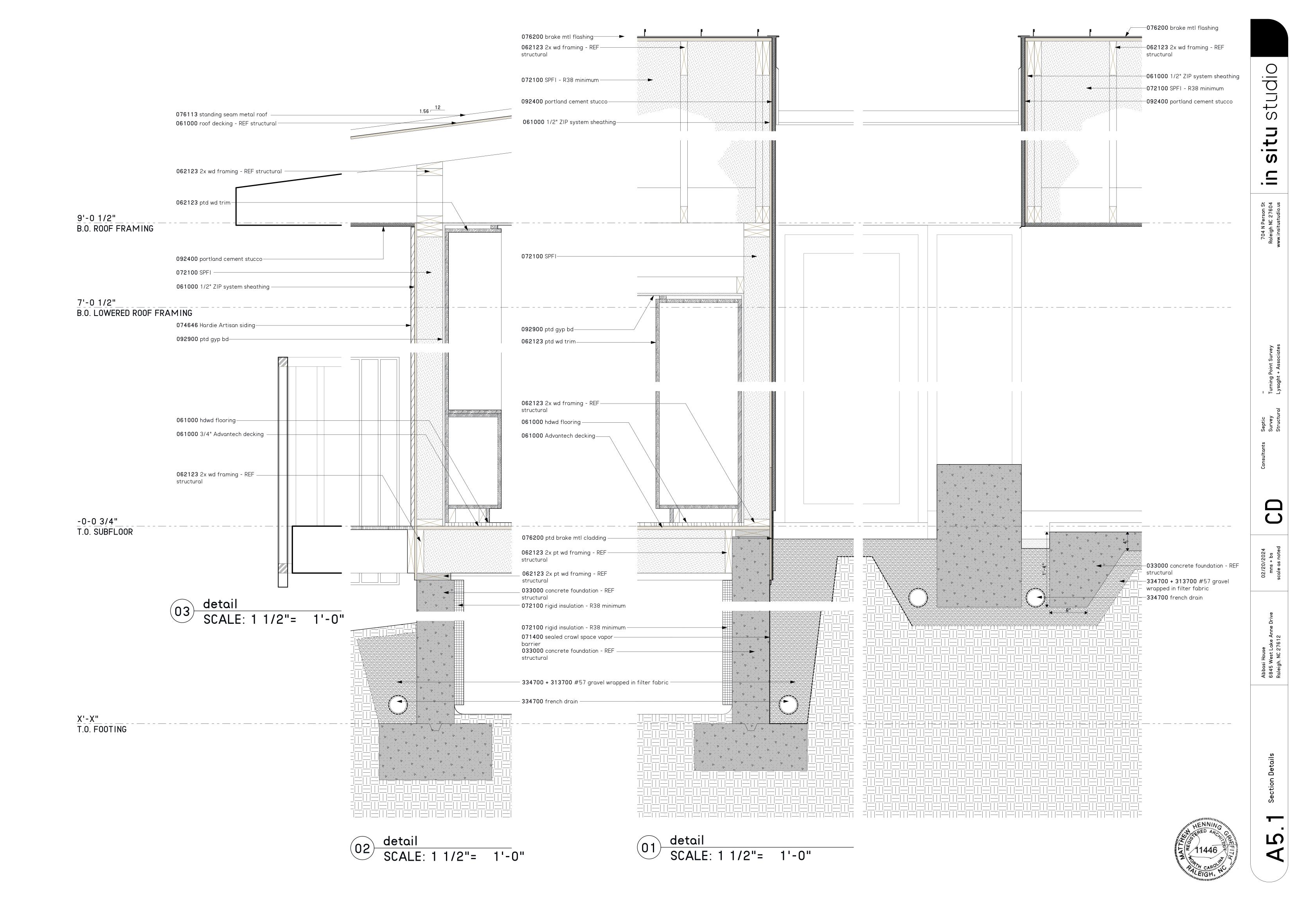
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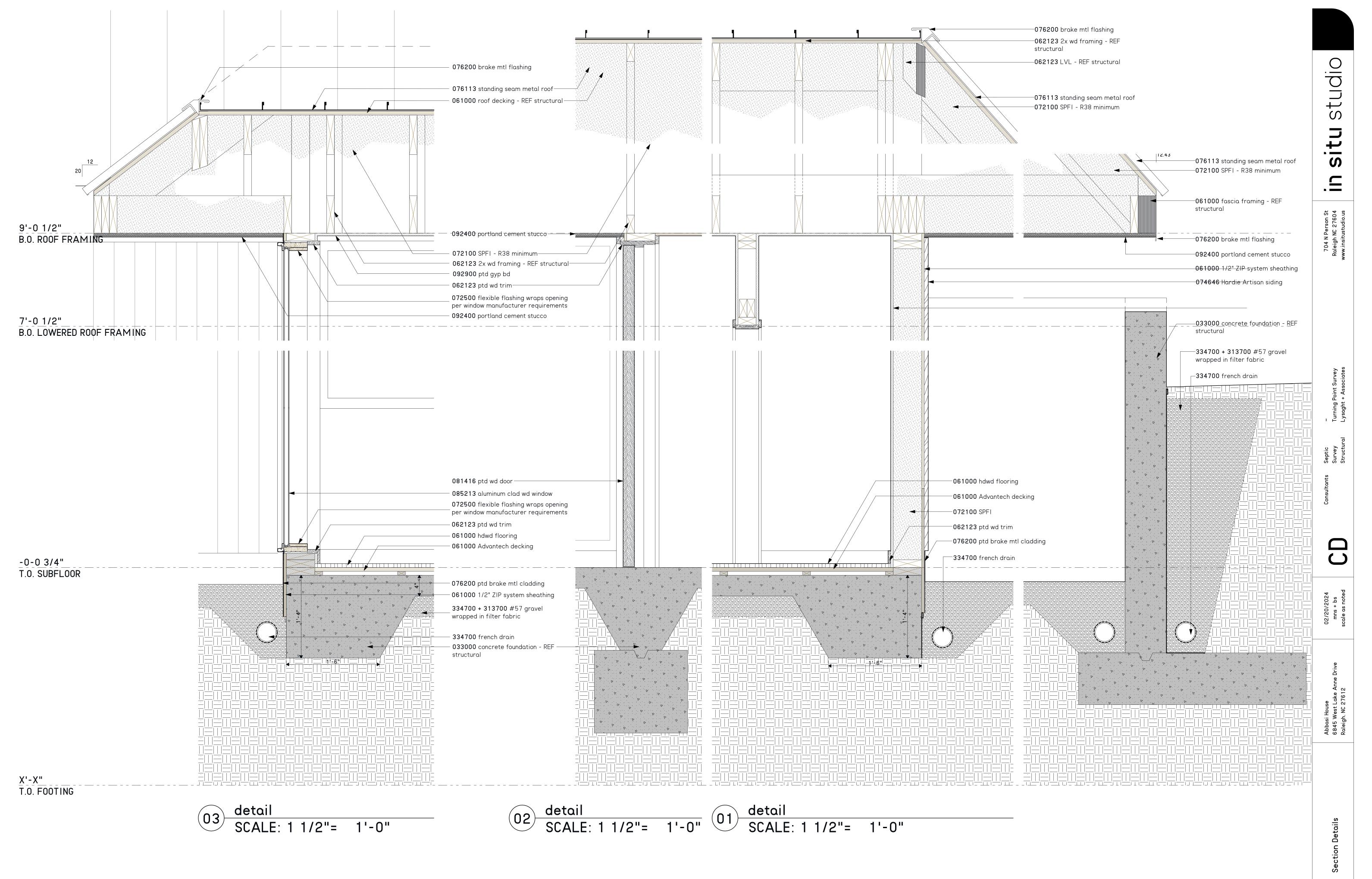
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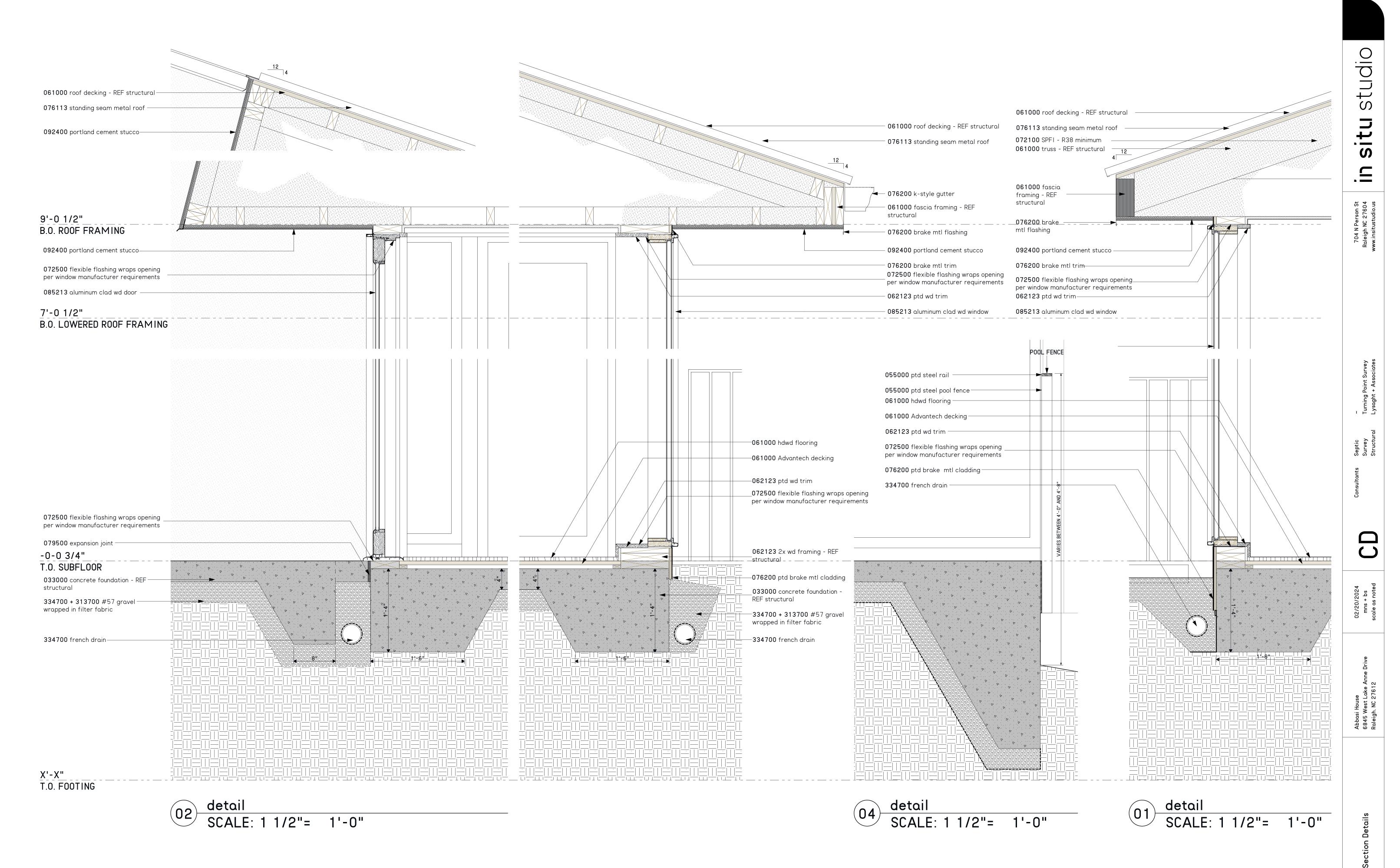
704 N Person St Raleigh NC 27604 www.insitustudio.us

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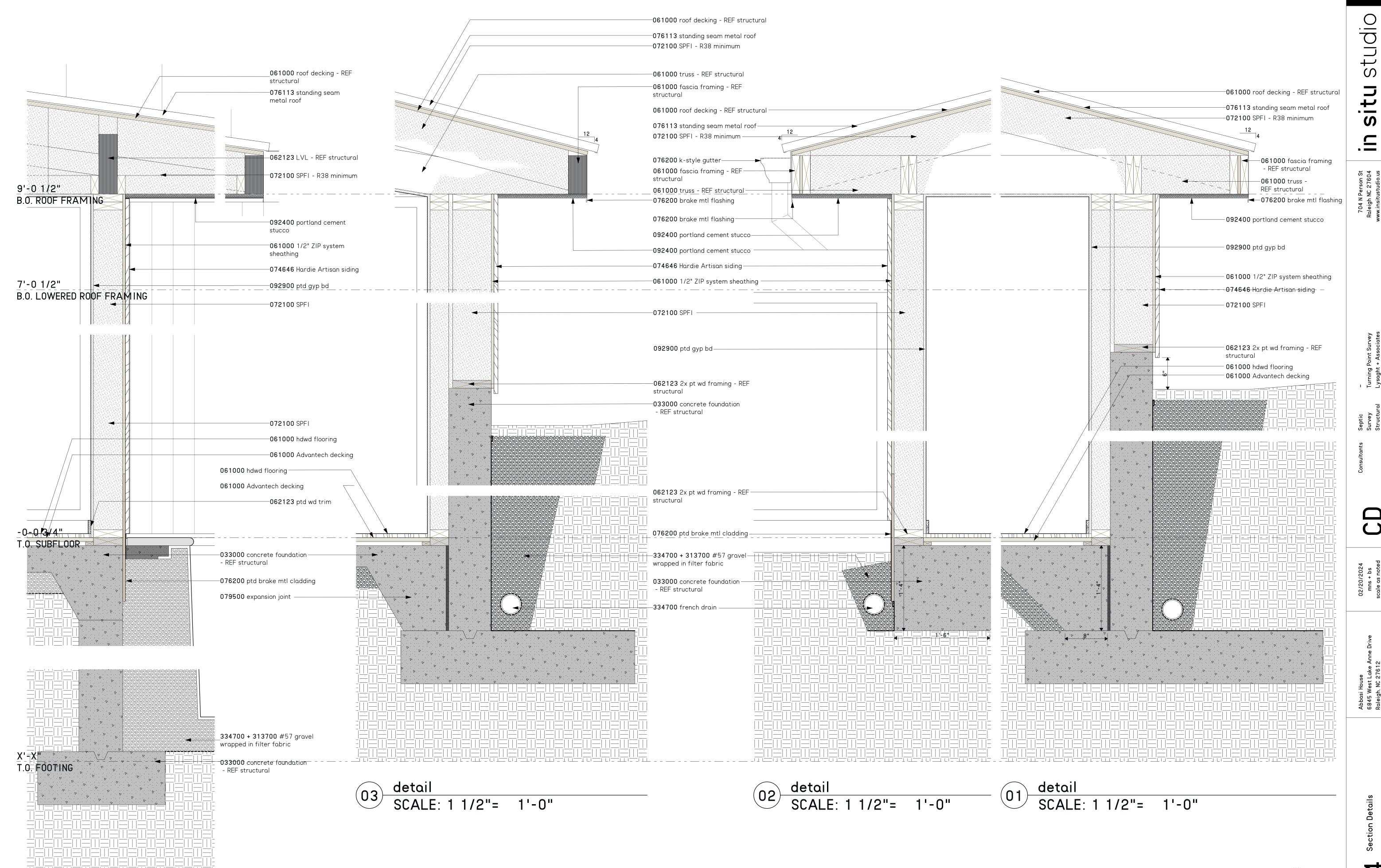






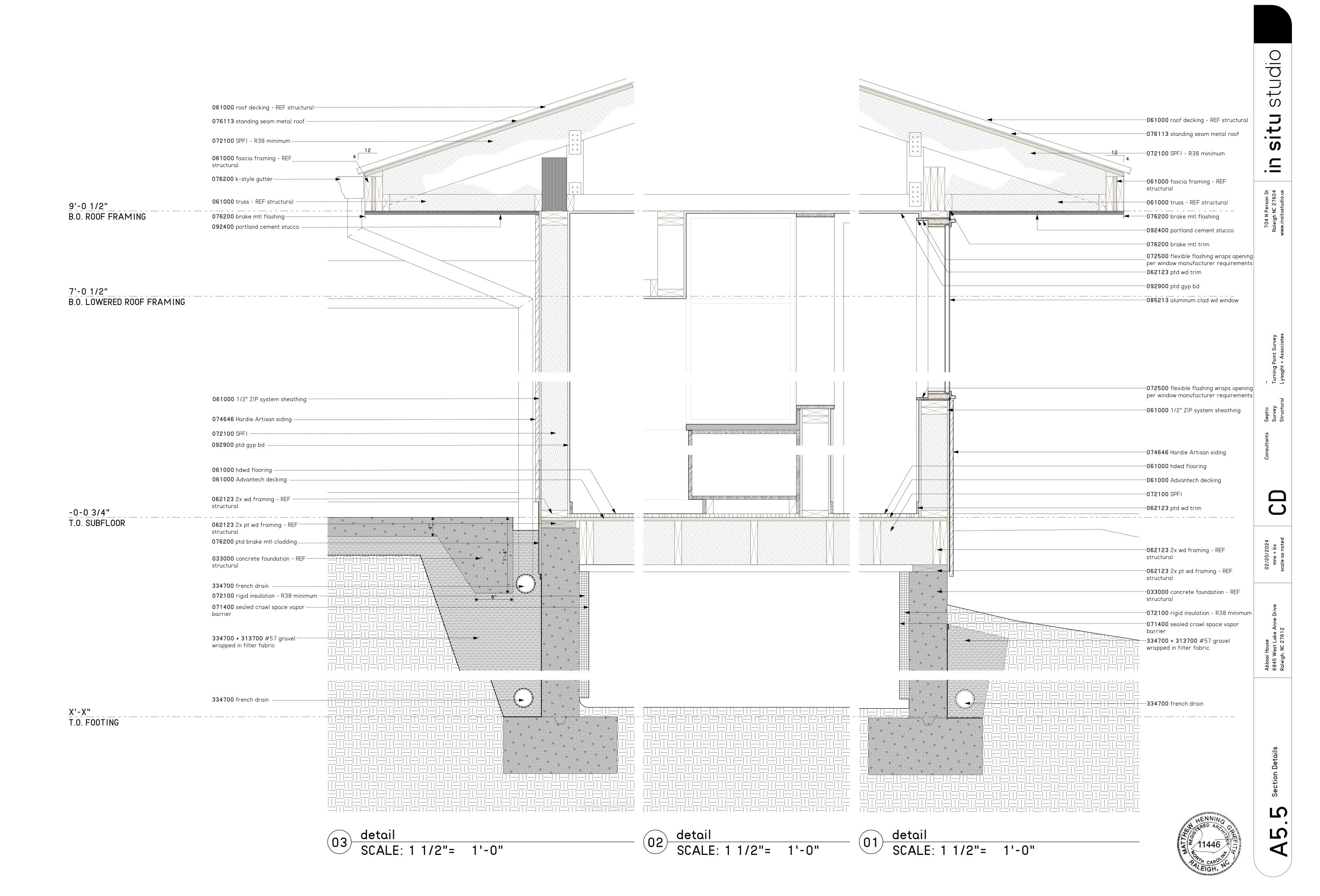


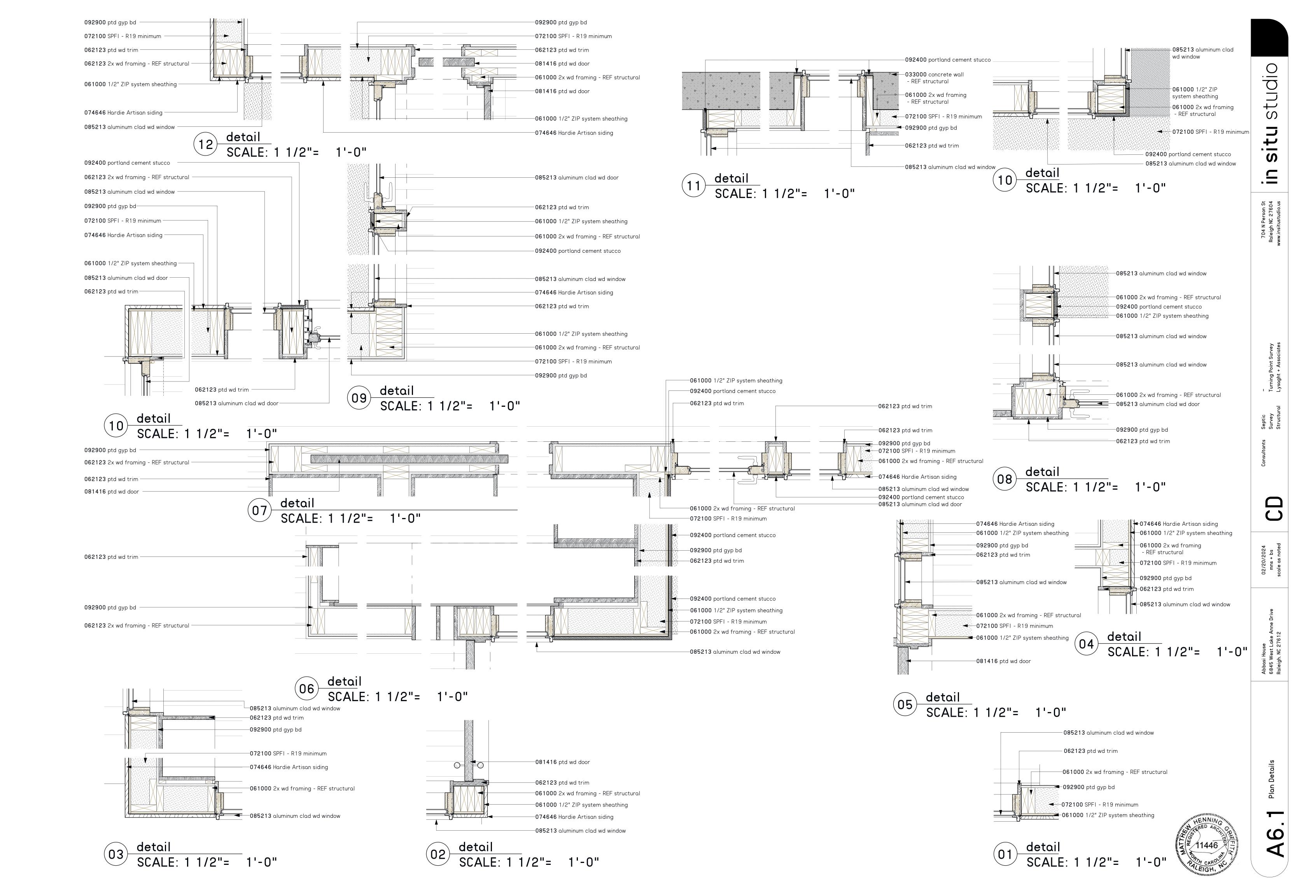




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04 detail
SCALE: 1 1/2"= 1'-0"





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#### **GENERAL STRUCTURAL NOTES**

#### **GENERA**

THESE DRAWINGS, AS INSTRUMENTS OF PROFESSIONAL SERVICE, ARE THE PROPERTY OF LYSAGHT & ASSOCIATES, P.A., FOR USE SOLELY WITH THIS PROJECT AND SHALL NOT BE REPRODUCED FOR OTHER PURPOSES.

THE PROFESSIONAL ENGINEER WHOSE SEAL APPEARS ON THESE DRAWINGS IS THE PROJECT STRUCTURAL ENGINEER-OF-RECORD (SER) WHO BEARS LEGAL RESPONSIBILITY FOR THE PERFORMANCE OF THE STRUCTURAL FRAMING RELATING TO PUBLIC HEALTH, SAFETY AND WELFARE. NO OTHER PARTY, WHETHER OR NOT A PROFESSIONAL ENGINEER, MAY COMPLETE, CORRECT, REVISE, DELETE OR ADD TO THESE CONSTRUCTION DOCUMENTS OR PERFORM INSPECTIONS OF THE WORK WITHOUT THE WRITTEN PERMISSION OF THE SER.

IN GENERAL, THE FOUNDATION AND FRAMING DETAILS FOR THIS PROJECT CAN BE CATEGORIZED AS "STANDARD RESIDENTIAL CONSTRUCTION" AND ARE TO BE WORKED OUT BY THE CONTRACTOR, IN THE FIELD. SPECIAL DETAILS ARE SHOWN ON THE DRAWINGS. IF ANY SPECIAL CONDITIONS ARISE THAT ARE NOT DETAILED ON THE DRAWINGS, CONTACT THE STRUCTURAL ENGINEER.

CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE "NORTH CAROLINA RESIDENTIAL CODE", 2018 EDITION.

ALL MEMBERS SHALL BE FRAMED, ANCHORED, TIED AND BRACED IN ACCORDANCE WITH GOOD CONSTRUCTION PRACTICE AND THE BUILDING CODE.

THE STRUCTURE SHOWN ON THESE DRAWINGS IS STRUCTURALLY SOUND ONLY IN ITS COMPLETED FORM. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY BRACING TO STABILIZE THE BUILDING DURING CONSTRUCTION.

#### **FLAT ROOF DRAINAGE**

FLAT ROOFS SHALL HAVE CONTROLLED DRAINAGE PROVISIONS AND SHALL BE EQUIPPED WITH A SECONDARY DRAINAGE SYSTEM AT A HIGHER ELEVATION WHICH PREVENTS PONDING ON THE ROOF ABOVE THAT ELEVATION. THE SECONDARY DRAINAGE SHALL BE SET SO THAT THE OVERFLOW SCUPPER IS 2" ABOVE THE ROOF AND A 6" MAXIMUM DEPTH OF WATER WILL POND ON THE ROOF, AT THE OVERFLOW SCUPPER, DURING THE DESIGN RAINSTORM. THE DESIGN OF THE ROOF DRAINAGE, SECONDARY DRAINAGE AND/OR OVERFLOW SCUPPERS IS BEYOND THE SCOPE OF THE STRUCTURAL ENGINEER'S SERVICES.

#### SCOPE OF STRUCTURAL ENGINEERING SERVICES

THE STRUCTURAL ENGINEER HAS PERFORMED THE STRUCTURAL DESIGN FOR THE NEW ADDITION AND REVIEWED THE ARCHITECTURAL PLANS FOR THIS PROJECT.

IF THE CONTRACTOR (OR OWNER) WOULD LIKE FOR CONSTRUCTION REVIEW SERVICES TO BE INCLUDED IN THE SCOPE AS AN ADDITIONAL SERVICE, THEN THE CONTRACTOR (OR OWNER) SHALL CONTACT THE STRUCTURAL ENGINEER AT THE FOLLOWING STAGES OF CONSTRUCTION FOR A FIELD REVIEW OF THE WORK:

- I. AFTER COMPLETION OF THE WOOD FRAMING SYSTEM, BEFORE INTERIOR FINISHES ARE INSTALLED.
- 2. AT ANY STAGE OF CONSTRUCTION WHEN DESIGN OR CONSTRUCTION PROBLEMS ARE ENCOUNTERED.

A "CONSTRUCTION REVIEW REPORT" WILL BE SENT TO THE CONTRACTOR AND THE ARCHITECT FOLLOWING EACH FIELD TRIP.

THE STRUCTURAL ENGINEER IS RESPONSIBLE FOR THE DESIGN OF THE PRIMARY STRUCTURAL SYSTEM FOR THE ADDITION. RESPONSIBILITY FOR ANY SECONDARY STRUCTURAL AND NON-STRUCTURAL SYSTEMS NOT SHOWN ON THE STRUCTURAL PLANS RESTS WITH THE CONTRACTOR.

THE STRUCTURAL ENGINEER HAS NOT DONE A SUBSURFACE INVESTIGATION (HE IS NOT A SOILS SPECIALIST). THE FOUNDATION DESIGN IS BASED UPON AN ASSUMED ALLOWABLE BEARING PRESSURE AS SHOWN IN THE "FOUNDATION" STRUCTURAL NOTES. THIS ALLOWABLE BEARING PRESSURE SHALL BE VERIFIED BY THE CONTRACTOR OR OWNER. IF PROBLEMS ARE ENCOUNTERED, A SOILS ENGINEER SHOULD BE RETAINED TO EVALUATE THE CONDITIONS AND RECOMMEND THE APPROPRIATE FOUNDATION SYSTEM.

THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR, AND WILL NOT HAVE CONTROL OF, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, OR FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE CONSTRUCTION WORK; NOR WILL HE BE RESPONSIBLE FOR THE CONTRACTOR'S FAILURE TO CARRY OUT THE CONSTRUCTION WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

FIELD MEASUREMENTS AND THE VERIFICATION OF DIMENSIONS SHOWN ON THE ARCHITECTURAL PLANS ARE NOT THE STRUCTURAL ENGINEER'S RESPONSIBILITY.

#### **ABBREVIATIONS**

•	• • •
ASD	ALLOWABLE STRESS DESIGN
B/U	BUILT-UP
CJ	CONTROL JOINT IN SLAB
CLNG	CEILING
COL	COLUMN
CEMH	CONCEALED FACE MOUNTS

CFMH CONCEALED FACE MOUNTED HANGER
DJ DOUBLE JOIST
DR DOUBLE RAFTER

FMH FACE MOUNTED HANGER
HD HOLD DOWN
LBW LOAD BEARING WALL
LVL LAMINATED VENEER LUMBER
NITS NOT TO SCALE

NTS NOT TO SCALE OC ON CENTER

SL PARALLEL STRAND LUMBER (PARALLAM)

PT PRESSURE TREATED

RyL LARGER OF TWO REACTIONS ON A BEAM, KIPS
RyS SMALLER OF TWO REACTIONS ON A BEAM, KIPS

SER STRUCTURAL ENGINEER-OF-RECORD S-P-F SPRUCE-PINE-FIR

S-P-F SPRUCE-PINE STD STANDARD

STL STEEL
SW SHEAR WALL

SYP SOUTHERN YELLOW PINE

TJ TRIPLE JOIST
TYP TYPICAL

UDH UPSIDE DOWN HANGER
UNO UNLESS NOTED OTHERWISE

#### **DESIGN LOADS**

ROOF DEAD LOAD	15	PSF
ROOF LIVE LOAD	20	PSF
DECK DEAD LOAD	10	PSF
DECK LIVE LOAD	40	PSF
FLOOR DEAD LOAD	15	PSF
FLOOR LIVE LOAD	40	PSF
ULTIMATE WIND SPEED (3 SECOND GUST)	115	MPH
EXPOSURE	В	

#### FOUNDATIONS

ALL FOOTINGS SHALL REST ON SOIL CAPABLE OF SAFELY SUPPORTING 2000 PSF. THE CONTRACTOR SHALL CONTACT THE STRUCTURAL ENGINEER IF UNSATISFACTORY SUBSURFACE CONDITIONS ARE ENCOUNTERED.

FOOTINGS SHALL BE CARRIED TO A LOWER ELEVATION THAN THOSE INICATED ON THESE DRAWINGS IF NECESSARY TO REACH FIRM UNDISTURBED SOIL.

FOUNDATIONS SHALL EXTEND NOT LESS THAN 12" BELOW THE FINISHED NATURAL GRADE OR ENGINEERED FILL IN NO CASE LESS THAN THE FROST LINE DEPTH.

ALL FILL SHALL BE PLACED IN 8" MAXIMUM LOOSE LIFTS AND SHALL BE COMPACTED TO A MINIMUM OF 95 PERCENT MAXIMUM DRY DENSITY AS DETERMINED IN ACCORDANCE WITH ASTM D-698 (STANDARD PROCTOR METHOD). THIS REQUIREMENT SHALL BE INCREASED TO 98 PERCENT OF ASTM D-698 IN THE FINAL FOOT BENEATH FLOOR SLABS AND PAVEMENTS.

THE SURFACE AREA ADJACENT TO THE FOUNDATION WALL SHALL BE PROVIDED WITH ADEQUATE DRAINAGE AND SHALL BE GRADED SO AS TO DRAIN SURFACE WATER AWAY FROM FOUNDATION WALLS.

WHEN TOP OR SUBSOILS ARE EXPANSIVE, COMPRESSIBLE OR SHIFTING, SUCH SOILS SHALL BE REMOVED TO A DEPTH AND WIDTH SUFFICIENT TO ASSURE STABLE MOISTURE CONTENT IN EACH ACTIVE ZONE AND SHALL NOT BE USED AS FILL.

#### CONCRETE

**MATERIALS** 

MINIMUM 28 DAY COMPRESSIVE STRENGTH OF CONCRETE SHALL BE 3000 PSI FOR FOOTINGS AND 4000 PSI FOR FOUNDATION WALLS. DO NOT CAST CONCRETE IN WATER OR ON FROZEN GROUND.

FOUNDATION WALLS, EXTERIOR WALLS, AND OTHER VERTICAL CONCRETE WORK EXPOSED TO THE WEATHER SHALL BE AIR ENTRAINED. TOTAL AIR CONTENT (PERCENT BY VOLUME OF CONCRETE) SHALL BE NOT LESS THAN 5 PERCENT OR MORE THAN 7 PERCENT.

#### SLAB-ON-GRADE CONSTRUCTION

CONCRETE SHALL BE DESIGNED TO MEET 4000 PSI COMPRESSIVE STRENGTH @ 28 DAYS AND EXHIBIT <0.04% SHRINKAGE @ 28 DAYS. THE MIX SHALL CONTAIN APPROXIMATELY 12 CUBIC FEET OF COARSE AGGREGATE (1½" TOP SIZE), THE SPECIFIED WATER REDUCING ADMIXTURE AND ACHIEVE A W/CM RATIO OF 0.53 (MAX.). INTERIOR SLABS SHALL NOT BE AIR-ENTRAINED; EXTERIOR SLABS SHALL BE AIR ENTRAINED.

<u>MIX</u>

< 0.04% @ 28 DAYS

517-560 LBS. CEMENT FLY ASH/SLAG **PROHIBITED COARSE AGGREGATE** 12 CU FT +/- .50 **FINE AGGREGATE** 7 CU FT +/- (ADJUST AS NECESSARY) TUF-STRAND FIBER 3 LBS/ CU YD WATER CONTENT 250 - 300 LBS. AIR CONTENT (ENTRAPPED AIR ONLY) 3.0% (MAX.) AT INTERIOR SLABS AIR CONTENT 4% - 6% AT EXTERIOR SLABS MID-RANGE WATER REDUCER (TYPE A/F) 3OZ.-10OZ./100WT +/-W/CM 0.53 (MAX.) INITIAL SLUMP (WATER) FINAL SLUMP 5.5" (MAX.)

CALCIUM CHLORIDE OR ADMIXTURES CONTAINING MORE THAN 0.05% CHLORIDE IONS ARE NOT PERMITTED. FLYASH, SLAG, AND BOTTOM ASH ARE NOT PERMITTED.

#### INTERIOR CURING

SHRINKAGE

MOISTURE RETAINING COVER: ALL INTERIOR CONCRETE SLABS SHALL BE PROTECTED FROM PREMATURE DRYING FOR A MINIMUM OF FIVE DAYS, AS REQUIRED IN ACI 301, USING MOISTURE-RETAINING COVER. FLOOD THE INTERIOR SLAB WITH SUFFICIENT WATER TO COVER THE SLAB. COVER CONCRETE SURFACES WITH MOISTURE-RETAINING COVER, PLACED IN WIDEST PRACTICAL WIDTH WITH SIDES AND ENDS LAPPED AT LEAST 3" AND SEALED BY WATERPROOF TAPE OR ADHESIVE. IMMEDIATELY REPAIR ANY HOLES OR TEARS DURING CURING PERIOD USING COVER MATERIAL AND WATERPROOF TAPE. REMOVE ANY AIR BUBBLES IN BETWEEN THE COVER AND THE INTERIOR SLAB. AFTER THE MINIMUM FIVE DAY CURING PERIOD, REMOVE MOISTURE-RETAINING COVER AND IMMEDIATELY SCRUB THE ENTIRE AREA

WITH AUTO-SCRUBBER AND INTERIOR CONCRETE FLOOR CLEANER. AFTER INTERIOR CONCRETE SLAB IS THOROUGHLY CLEANED OF ALL SALTS, LAITANCE, DIRT AND DEBRIS, ALLOW DRYING FOR AT LEAST SIX (6) HOURS.

#### **EXTERIOR CURING AND SEALING**

ASTM C1315, TYPE I, CLASS B, (700G/L): LIQUID TYPE MEMBRANE-FORMING CURING COMPOUND, CLEAR STYRENE ACRYLATE TYPE, COMPLYING WITH ASTM C1315, TYPE I, CLASS B, 25% SOLIDS CONTENT MINIMUM. MOISTURE LOSS SHALL BE NOT MORE THAN 0.30 KG/M2 WHEN APPLIED AT 300 SQ. FT./GAL. MANUFACTURER'S CERTIFICATION IS REQUIRED. ACCEPTABLE PRODUCTS: "SUPER REZ SEAL" BY EUCLID CHEMICAL OR "KURE N SEAL 30" BY BASF.

PLACE FLOOR SLAB ON A WELL COMPACTED BASE. THE SUBGRADE SHALL BE GRANULAR, NON-EXPANSIVE SOIL (THAT IS, WITHOUT CLAY), WHICH HAS BEEN COMPACTED TO AT LEAST 95% AND VERIFIED BY ON-SITE TESTING.

CONCRETE STRENGTH SHALL BE 4000 PSI AT 28 DAYS. USE A WATER REDUCING ADMIXTURE TO REDUCE WATER, INCREASE WORKABILITY AND DECREASE SHRINKAGE CRACKS.

PROVIDE ISOLATION JOINTS IN SLABS AS FOLLOWS:

- I. BETWEEN SLABS ON GRADE AND FOUNDATION WALLS,
- 2. BETWEEN SLABS AND INSERTS SUCH AS PIPES,
- 3. AT JUNCTION OF GARAGE SLAB AND DRIVEWAY,
- 4. AROUND STEEL COLUMNS AT SPREAD FOOTINGS,

THE CONTROL JOINT SPACING SHALL BE APPROXIMATELY 12' FOR A 4" THICK SLAB.
PLACE CONTROL JOINTS TO AVOID REENTRANT CORNERS. MAKE SAWCUTS TO FORM
WEAKEN PLANE CONTROL JOINTS AS SOON AS POSSIBLE.

LIGHTLY DAMPEN THE SUBGRADE BEFORE PLACING CONCRETE TO PREVENT THE SUBGRADE FROM ABSORBING WATER FROM CONCRETE MIX. APPLY WATER AT NEARLY THE SAME RATE IT SOAKS INTO THE SUBGRADE SURFACE.

SEE ARCHITECTURAL FOR SLAB FINISHING REQUIREMENTS.

DURING HOT WEATHER, USE A FOG SPRAY TO KEEP THE SURFACE DAMP BEFORE

START CURING AS SOON AS THE FINISHERS ARE DONE.

#### **REINFORCING STEEL**

ALL DETAILING, FABRICATION AND PLACING OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH THE LATEST "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES," ACI 315.

REINFORCING BARS SHALL BE NEW BILLET STEEL CONFORMING TO ASTM A615, GRADE 60. CLEAR CONCRETE COVER OVER BARS SHALL BE 3" FOR FOOTINGS.

PROVIDE CORNER BARS AT ALL FOOTING STEPS AND CORNERS. BARS SHALL BE A MINIMUM OF 2'-6" LONG AND SHALL HAVE THE SAME SIZE AND SPACING AS HORIZONTAL REINFORCING.

LAP ALL SPLICES AS SPECIFICALLY CALLED FOR, BUT AT LEAST 36 BAR DIAMETERS IN CONCRETE, UNLESS NOTED OTHERWISE.

PROVIDE DOWELS IN WALL FOOTINGS EQUIVALENT IN SIZE AND NUMBER TO VERTICAL STEEL EXTENDING 16 BAR DIAMETERS INTO FOOTING AND 36 BAR DIAMETERS INTO WALL, UNLESS NOTED OTHERWISE.

#### STRUCTURAL STEEL

INTERIOR STRUCTURAL STEEL SHALL RECEIVE ONE SHOP COAT OF RUST INHIBITIVE PAINT. EXTERIOR STRUCTURAL STEEL SHALL BE HOT DIP GALVANIZED OR EPOXY PAINTED. STEEL BELOW GRADE SHALL BE BOTH GALVANIZED AND EPOXY PAINTED.

THE STEEL USED SHALL HAVE THE FOLLOWING MINIMUM YIELD STRESS:
WIDE FLANGE SHAPES 50 KSI
STRUCTURAL TUBE COLUMNS 46 KSI
MISCELLANEOUS SHAPES 36 KSI

USE 3/4" DIAMETER A-325 BOLTS FOR ALL STEEL TO STEEL CONNECTIONS U.N.O. USE 3/4" DIAMETER A-307 BOLTS FOR ALL ANCHOR BOLTS U.N.O. USE E-70 ELECTRODES FOR ALL SHOP AND FIELD WELDING.

#### WOOD FRAMING

EXTERIOR WALLS STUDS, FLOOR JOISTS AND ROOF JOISTS SHALL BE #2 GRADE S-P-F UNLESS NOTED OTHERWISE ON THE DRAWINGS.

NONBEARING INTERIOR STUDS MAY BE UTILITY GRADE LUMBER.

SUBFLOOR SHALL BE 3/4" T&G PLYWOOD WITH A 48/24 APA RATING. USE SOUTHERN PINE, CDX OR STRUCTURAL EQUIVALENT.

HEADERS OVER OPENINGS IN LOAD BEARING WALLS SHALL BE AS SHOWN AT THE "HEADER SCHEDULE" DETAIL.

USE LVL FOR ALL FLITCH BEAMS AND A36 STEEL FOR FLITCH PLATE. ATTACH THE MEMBERS TOGETHER WITH 5/8" DIAMETER BOLTS @ 16" O.C. STAGGERED, AND DOUBLE BOLTS AT BOTH ENDS. PROVIDE CONTINUOUS LATERAL SUPPORT FOR TOP OF BEAM. DO NOT SPLICE LVL BEAMS BETWEEN SUPPORT POINTS.

LVL BEAMS AND HEADERS THAT ARE DOUBLED SHALL BE NAILED TOGETHER WITH 2 ROWS OF 16d NAILS @ 12" O.C. STAGGERED. PROVIDE CONTINUOUS LATERAL SUPPORT FOR TOP OF HEADER. STRENGTH OF LVL BEAMS AND HEADERS SHALL BE EQUAL TO THAT PROVIDED BY TRUS JOIST: Fy = 285 PSI, Fb = 2600 PSI, E = 1900 KSI.

BUILT-UP STUD COLUMNS SHALL BE SECURELY NAILED TOGETHER TO ACT AS A COMPOSITE MEMBER. USE (2) 12d NAILS FOR EACH STUD AT 9" O.C. WITH NAILS INSTALLED ON ALTERNATE SIDES OF COLUMN.

THE HEIGHT OF STUD BEARING WALLS IS LIMITED TO 10' BETWEEN LATERAL BRACING UNLESS NOTED OTHERWISE BY STRUCTURAL ENGINEER. CONTACT STRUCTURAL ENGINEER FOR STUD HEIGHTS GREATER THAN 10'-0". STUDS SHALL NOT BE SPLICED AT TALL WALLS, EXCEPT AT POINTS OF LATERAL SUPPORT.

ANY WOOD EXPOSED TO THE ELEMENTS, OR IN CONTACT WITH MASONRY, SHALL BE PRESERVATIVE TREATED TO THE RETENTIONS SHOWN IN THE BUILDING CODE.

OUTDOOR DECKS SHALL BE CONSTRUCTED IN ACCORDANCE WITH APPENDIX M OF THE BUILDING CODE.

#### LIGHT GAGE SIMPSON CONNECTIONS

SOME PRESERVATIVE TREATED WOOD HAS A CORROSIVE EFFECT ON LIGHT GAGE CONNECTIONS. USE TYPE 304 OR 316 STAINLESS STEEL UNLESS GALVANIZED CONNECTORS ARE SPECIFICALLY RECOMMENDED BY THE TREATED WOOD SUPPLIER.

#### STRUCTURAL REQUIREMENTS IN 115 MPH WIND ZONE

FOUNDATIONS IN THE 115 MPH WIND ZONE SHALL BE AT LEAST 10" DEEP X 24" WIDE, REINFORCED WITH (3) #4'S LOCATED 3" ABOVE THE BOTTOM OF FOOTING. THE REBARS SHALL BE CONTINUOUS WITH 18" MINIMUM LAPS AT SPLICES AND CORNERS.

PRESERVATIVE TREATED WOOD SILLS ON CONTINUOUS FOUNDATION WALLS SHALL BE ANCHORED WITH 1/2" BOLTS WITH 2 X 2 X 1/8 WASHERS SPACED NOT MORE THAN 4'-0" APART AND WHICH ARE EMBEDDED AT LEAST 8" IN CONCRETE OR 16" IN MASONRY UNITS. INSTALL TWO ANCHOR BOLTS WITHIN 6" OF THE CORNERS OF THE BUILDING, AT EACH DOOR AND WINDOW JAMB AND WITHIN 12" OF EACH END AT SILL SPLICES.

INSTALL THREE STUDS (MIN) AT EVERY CORNER OF AN EXTERIOR WALL.

ALL EXTERIOR WALLS SHALL BE FULLY SHEATHED WITH 7/16" STRUCTURAL SHEATHING TO PROVIDE LATERAL STRENGTH FOR WIND LOADS AND TO PROVIDE A CONTINUOUS TIE FROM ROOF DOWN TO THE FOUNDATION WALL. SHEATHING SHALL BE ATTACHED TO THE STUDS WITH 8d NAILS AT 4" O.C. ALONG THE PANEL EDGES AND 12" O.C. AT INTERMEDIATE LOCATIONS. BLOCK BETWEEN STUDS AT PLYWOOD JOINTS.

EACH ROOF JOIST SHALL BE ATTACHED TO THE EXTERIOR WALL WITH A SIMPSON HURRICANE TIE.

WALL BRACING HAS BEEN DESIGNED TO COMPLY WITH SECTION R301.1, SO IT MEETS THE REQUIREMENTS OF R602.10.

# SIMPSON HANGER SCHEDULE SIZE HANGER ALLOW. LOAD 2 × 8 LUS26, LUC26Z 710# (2) 2 × 8 HU26-2, HUC26-2 I190# (3) 2 × 8 HU26-3, HUC28-2 I190#

(3) 2 X 8 2 X I0 LUS210, LUC210Z 1150# (2) 2 X I 0 HU210-2, HUC210-2 2085# (3) 2 X 10 HU210-3, HUC210-3 2085# 2 X I2 .US210, LUC210Z 1150# (2) 2 X 12 HU212-2, HUC212-2 2385# (3) 2 X I2 HU212-3, HUC212-3 2385# 3 1/2 X 9 1/4 (9 1/2) LVL OR PSL HGUS410, HUCQ410-SDS 9100#, 4500# 5 1/4 X 9 1/4 (9 1/2) LVL OR PSL HGUS5.5/10, HUCQ610-SDS 5635#, 4680# 3 1/2 X 11 1/4 (11 7/8) LVL OR PSL HGUS412, HUCQ412-SDS 9400#, 5045# 5 1/4 X 11 1/4 LVL OR PSL HGUS5.5/12, HUCQ612-SDS 9400#, 5185# 9695#, 5045#

3 1/2 X 14 LVL OR PSL HGUS414, HUCQ414-SDS

5 1/4 X 14 LVL OR PSL HGUS5.5/14, HUCQ614-SDS

3 1/2 X 16 LVL OR PSL HGUS414, HUCQ414-SDS

5 1/4 X 16 LVL OR PSL HGUS5.5/14, HUCQ614-SDS

#### NOTES

I. LOAD VALUES SHOWN IN THE TABLE ABOVE DO NOT INCLUDE THE LOAD DURATION FACTOR. DO NOT USE CONCEALED HANGERS EXCEPT WHERE SPECIFICALLY CALLED OUT ON DRAWINGS.

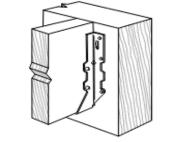
2. USE HANGER PER SCHEDULE ABOVE UNLESS SPECIFIED DIFFERENTLY ON FRAMNG PLAN. ALL FLUSH WOOD/WOOD CONNECTIONS SHALL BE MADE WITH HANGERS. OTHER HANGERS MAY BE SUBSTITUTED FOR THOSE SHOWN IF DESIGN VALUES ARE EQUAL TO OR GREATER THAN THOSE IN THE TABLE.

3. INSTALL HANGERS PER MANUFACTURER'S SPECIFICATIONS.

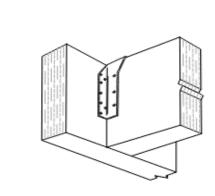
4. USE STAINLESS STEEL HANGERS IF EXPOSED TO THE ELEMENTS OR IN CONTACT WITH TREATED WOOD. (GALVANIZED HANGERS MAY BE USED IN LIEU OF STAINLESS STEEL IF SPECIFICALLY RECOMMENDED BY SIMPSON AND THE TREATING COMPANY.)

5. FACE (OR TOP) MOUNTED HANGERS FOR WOOD TRUSSES AND I-JOISTS SHALL BE DESIGNED AND FURNISHED BY THE SUPPLIERS.

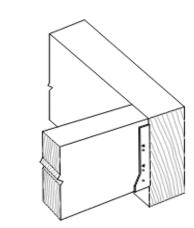
6. AT SPECIAL CONDITIONS - SKEWED, PITCHED, ETC. - CONTRACTOR SHALL SELECT HANGER WITH AN ALLOWABLE LOAD GREATER THAN REACTION SHOWN IN SCHEDULE FOR FACE MOUNTED (NOT CONCEALED) HANGER.



TYPICAL FACE MOUNTED HANGER



TYPICAL UPSIDE DOWN HANGER

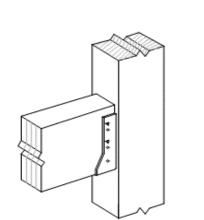


9695#, 5185#

9695#, 5045#

9695#, 5185#

TYPICAL CONCEALED HANGER



TYPICAL CONCEALED HANGER TO POST





#### CONCRETE FOUNDATION WALL NOTES

- I USE CLASS "A" SOIL FOR BACKFILL BEHIND WALL TO MINIMIZE LATERAL PRESSURE. CLASS "A" IS CLEAN SAND OR GRAVEL, FREE OF FINES THAT MIGHT OBSTRUCT FREE DRAINAGE. BACKFILL MUST BE APPROVED BY THE GEOTECHNICAL ENGINEER.
- STRUCTURAL DATA: f'c = 3,000 PSI FOR FOOTINGS, f'c = 4,000 PSI FOR WALLS, GRADE 60 REBARS, 2,000 PSF ALLOWABLE SOIL BEARING PRESSURE, 40 PCF EQUIVALENT FLUID PRESSURE AT FOUNDATION WALLS.
- 3 NEW FOUNDATION WALLS ARE 8" THICK. REINFORCE WITH #4
  VERTICALS AT 18" O.C. AND #4 HORIZONTALS AT 18" O.C. REBARS
  ARE LOCATED AT MID-DEPTH OF WALL.
- 4 PROVIDE WEAKENED PLANE CONTRACTION JOINTS AT INTERVALS OF ABOUT 25 FEET AND KEYED EXPANSION JOINTS AT EVERY FOURTH CONTRACTION JOINT. CUT ALTERNATE LONGITUDINAL BARS EXACTLY OPPOSITE WEAKENED PLANE JOINTS.
- 5 REFER TO ARCHITECTURAL PLANS AND/OR SPECIFICATIONS FOR WATER- PROOFING REQUIREMENTS AND DRAINAGE REQUIREMENTS.
- 6 THE MAXIMUM SIZE AGGREGATE FOR WALLS IS 3/4".
- 7 IF THE VERTICAL BARS ARE SPLICED, USE 36 BAR DIAMETERS

#### FLOOR FRAMING NOTES

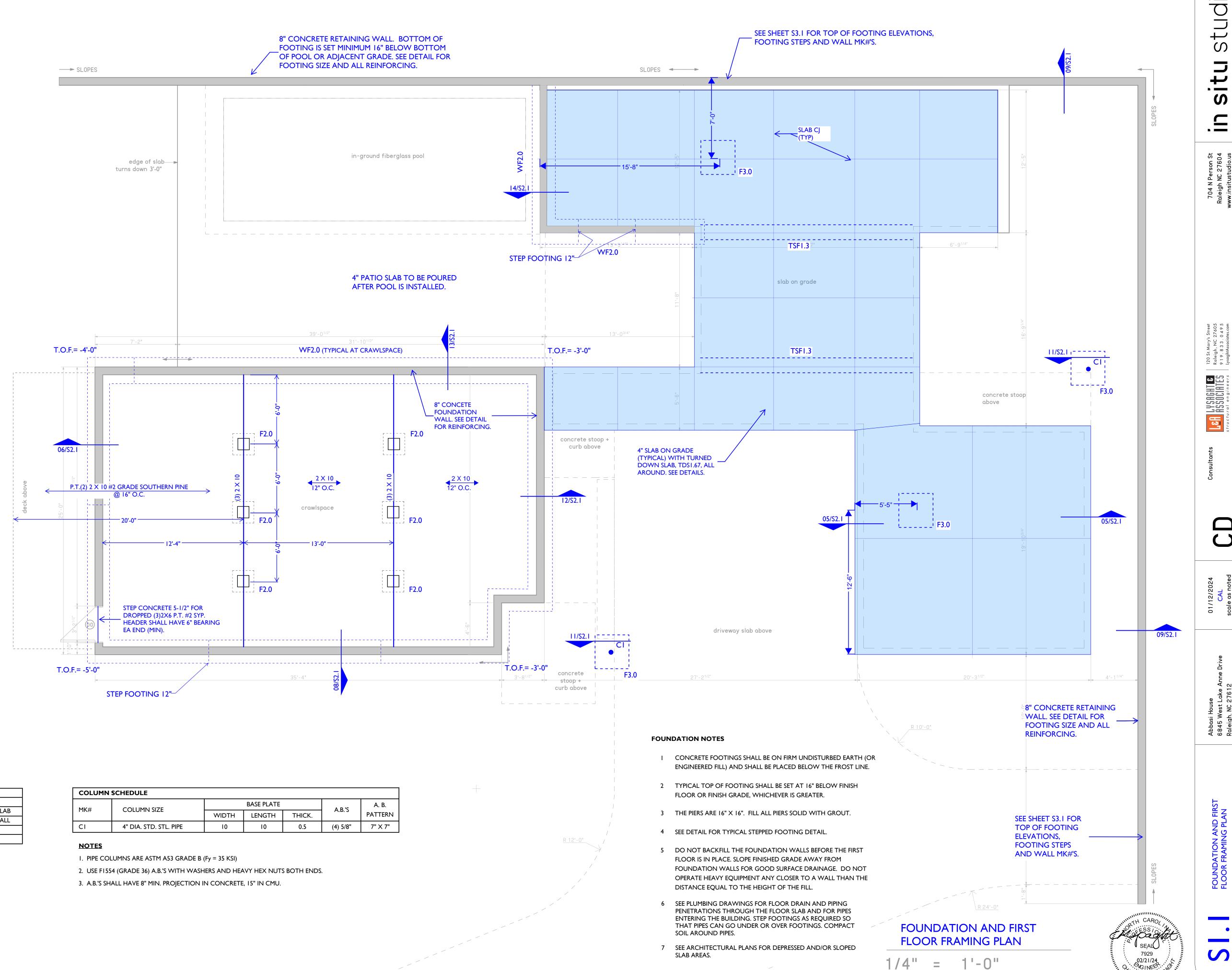
I REFER TO ARCHITECTURAL PLANS FOR ALL DIMENSIONS NOT SHOWN ON STRUCTURAL DRAWINGS.

- 2 SUBFLOOR SHALL BE 3/4" T&G PLYWOOD WITH A 40/24 APA RATING.
  USE SOUTHERN PINE, CDX OR STRUCTURAL EQUIVALENT.
- 3 COORDINATE OPENINGS IN THE FLOOR FRAMING WITH THE ARCHITECTURAL AND MECHANICAL DRAWINGS. OPENINGS LARGER THAN 6" BUT LESS THAN 14" SHALL BE FRAMED ON ALL SIDES WITH 2 X 4 HEADERS. CONTACT STRUCTURAL ENGINEER FOR OPENINGS GREATER THAN 14" WIDE.
- 4 WOOD SILLS SHALL BE ATTACHED TO CONTINUOUS FOUNDATION WALLS AS SPECIFIED IN THE GENERAL STRUCTURAL NOTES.
- 5 ALL EXTERIOR WALLS ARE LOAD BEARING. INTERIOR LOAD BEARING WALLS ARE SHADED. JOISTS MAY BE SPLICED OVER LOAD BEARING WALLS, BUT SHALL NOT BE SPLICED OVER NON-LOAD BEARING WALLS.
- 6 FLUSH HEADER TO HEADER CONNECTIONS SHALL BE WITH STEEL HANGERS. SEE DETAILS.
- 7 USE A DOUBLE JOIST UNDER ALL NON-LOAD BEARING WALLS THAT RUN PARALLEL TO THE JOISTS. USE (2) SETS OF DOUBLE JOISTS UNDER BATHTUBS TO CARRY THE EXTRA WEIGHT OF THE TUB.
- 8 SEE HEADER SCHEDULE FOR SIZES OF MEMBERS DENOTED ON FRAMING PLAN AS H1, H2, H3, ETC.

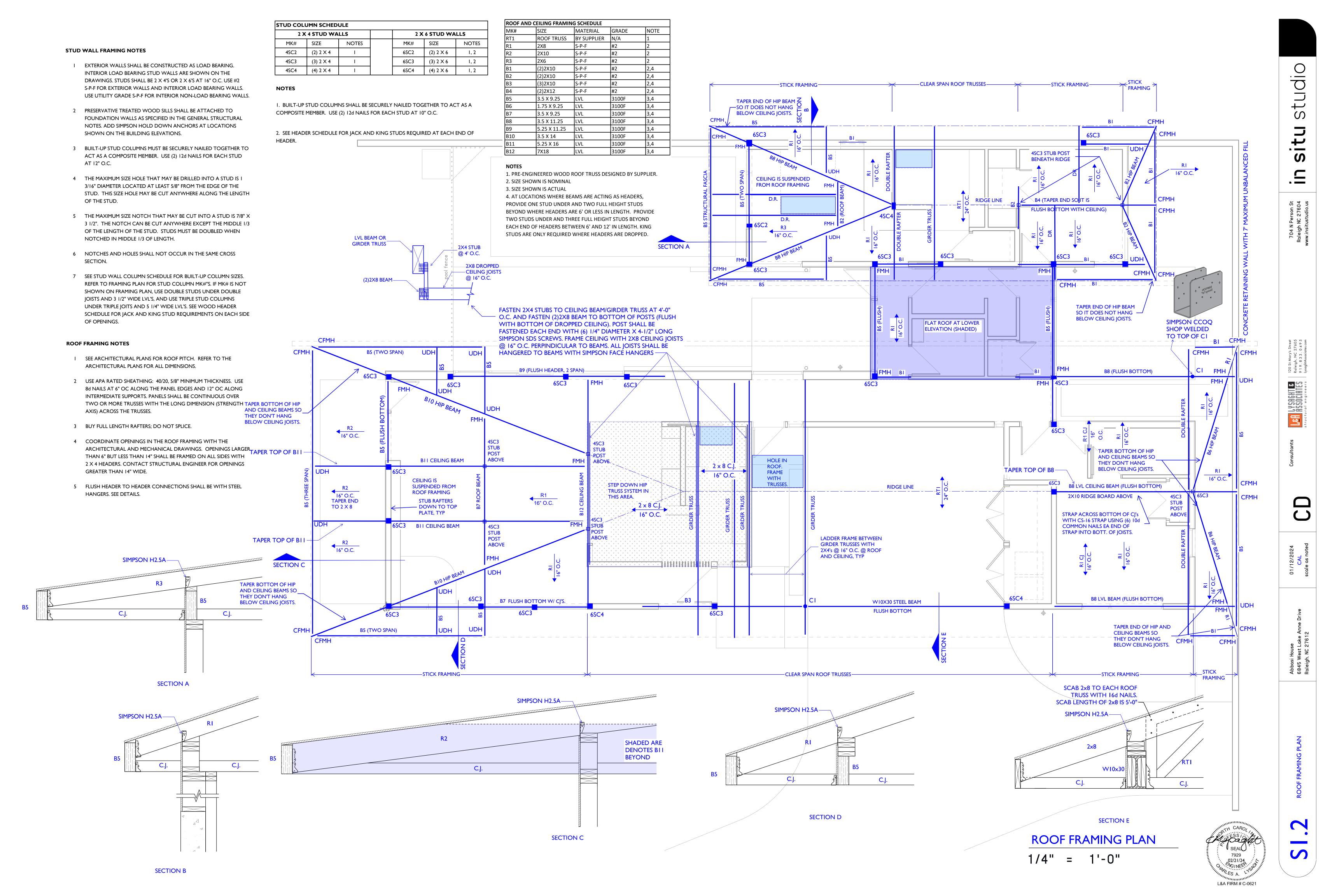
FOOTING SCHEDULE			
MK#	SIZE	REINFORCING	NOTES
TDS1.67	I'-6" WIDE X 16" DEEP	(2) #4 CONTINUOUS	AT TURNDED DOWN SLAB
WF2.0	2'-0" WIDE X 10" THICK	(3) #4 CONTINUOUS	AT 8" FOUNDATION WALL
F2.0	2'-0" × 2'-0" × 10"	UNREINFORCED	PIER FOOTING
F3.0	3'-0" × 3'-0" × 10"	(4) #4 EACH WAY	COLUMN FOOTING

#### FOOTING NOTES

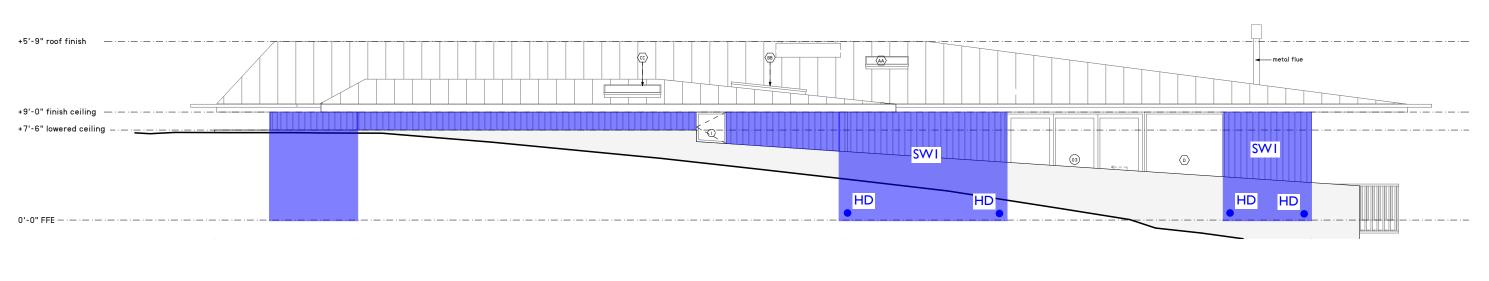
I. REINFORCING TO BE LOCATED 3" CLEAR FROM BOTTOM OF FOOTING.



L&A FIRM # C-0621

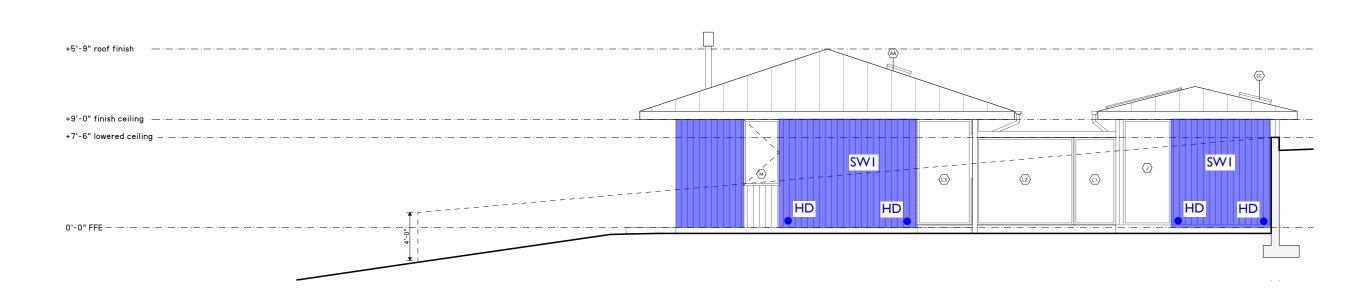


SCALE: 1/8" = 1'-0"



NORTH ELEVATION SHEAR WALL LAYOUT

SCALE: 1/8" = 1'-0"



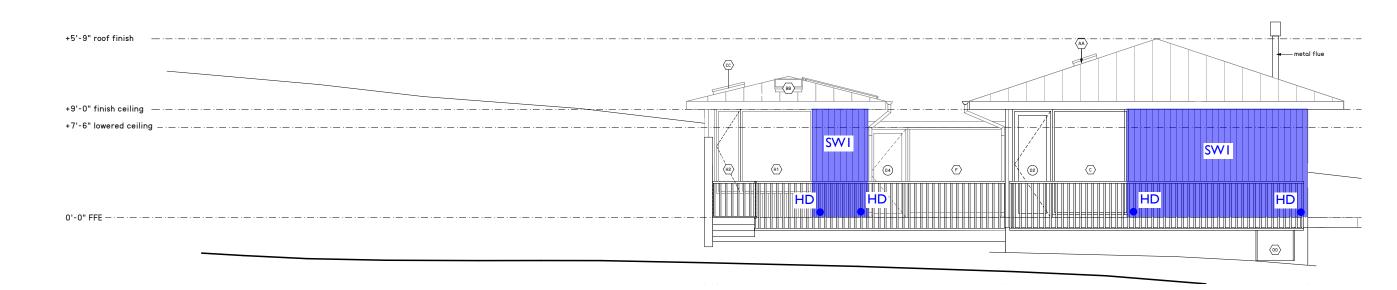
EAST ELEVATION SHEAR WALL LAYOUT

SCALE: 1/8" = 1'-0"



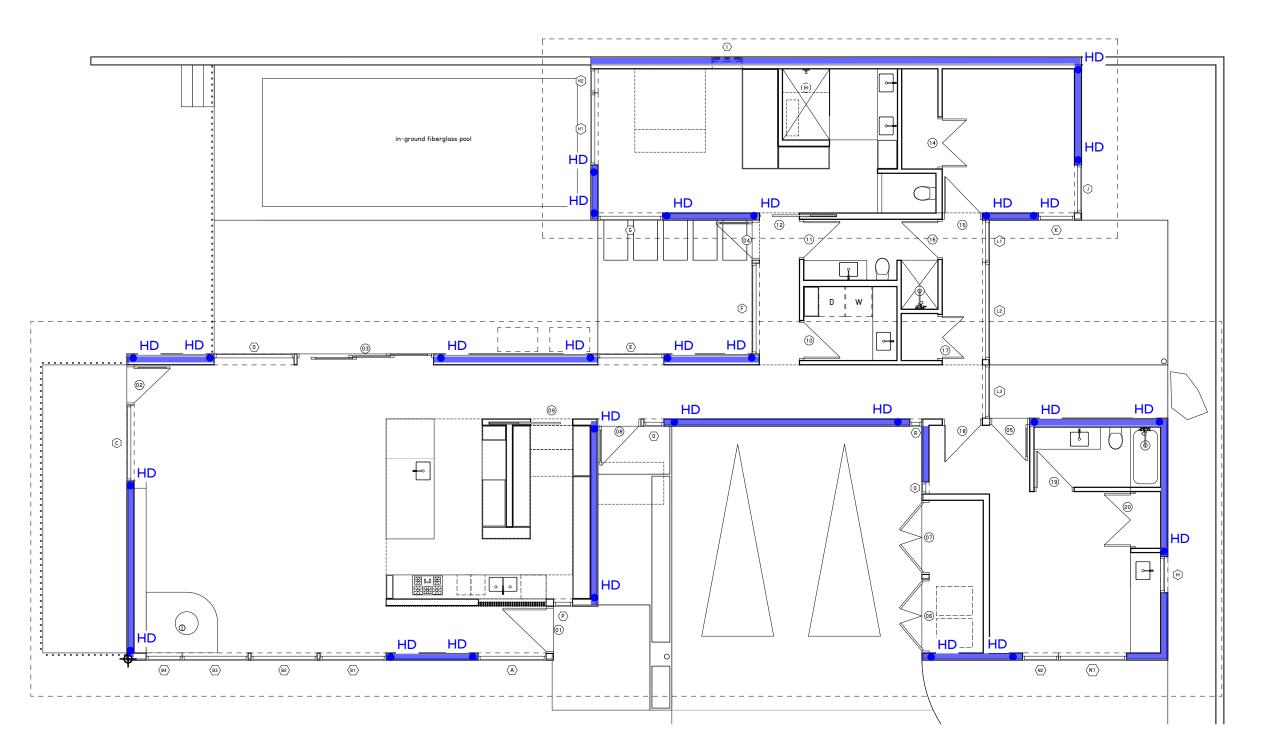
SOUTH ELEVATION SHEAR WALL LAYOUT

SCALE: 1/8" = 1'-0"



WEST ELEVATION SHEAR WALL LAYOUT

SCALE: 1/8" = 1'-0"



PLAN VIEW SHEAR WALL LAYOUT



HD INDICATES SIMPSON "HDU8-SDS2.5" HOLD DOWN ANCHOR TO BE INSTALLED W/ 7/8" ANCHOR BOLT. ANCHOR TO BE FASTENED TO (3)2X4 STUD COLUMN WITH (20) 1/4" X 2.5" SDS WOOD SCREWS.

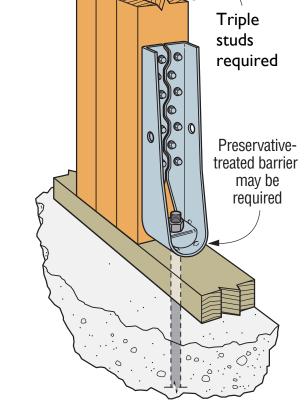
**SHEAR WALL FRAMING NOTES** 

I. 7/16" PLYWOOD, OSB OR STRUCTURALLY EQUIVALENT ZIP SHEATHING ARE SHADED BLUE AND PROVIDE OVERALL STABILITY TO THE BUILDING.

2. ATTACH SHEATHING TO STUDS WITH 10d NAILS AT 4" O.C. AROUND PERIMETER OF SHEET AND 12" O.C. AT INTERMEDIATE STUDS. BLOCK ALL HORIZONTAL JOINTS IN SHEATHING.

3. EXTEND SHEATHING UP TO UNDERSIDE OF PLYWOOD ROOF SHEATHING. NAIL ROOF SHEATHING TO TOP OF STUD WALL WITH 10d NAILS AT 4" O.C.

4. SEE LOCATIONS ON ELEVATIONS AND PLANS FOR HOLD DOWNS (HD) AND TYPICAL HOLD DOWN DETAIL THIS SHEET.



Vertical HDU Installation **TYPICAL HOLD DOWN DETAIL** 

